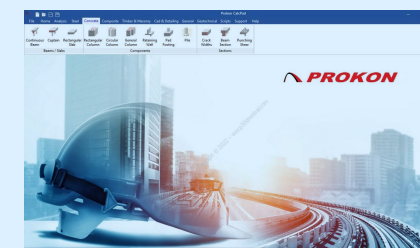
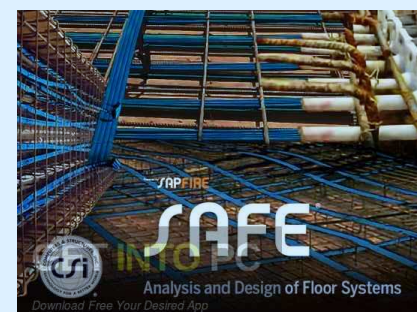


THE COPYRIGHTS OF THESE PLANS AND DRAWINGS ARE RESERVED  
FOR  
DR-MAJID AL BANA



PREPARE BY **DR-Majid Albana**  
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Notes

THE BUILDING SYSTEM WILL BE  
CONSIDER AS SHEAR WALL  
BUILDING WITH COLUMNS AND  
THE SLAB WILL BE AS FLAT SLAB  
WITH DROP PANEL.THE  
SOFTWARE USED IN DESIGN (CSI  
ETABS 2022, AND CSI SAFE  
2022&PROKON) IS THE GENERAL  
PROGRAM USED IN THIS DESIGN

job title

**MALL BUILDING**

Structural  
Drawings

DRWG. TITLE:

DESIGNED BY **DR-Majid Albana**

CHECKED BY

SCALE **As Shown**

DATE **10 /2023**

SHEET NO **Str 1**

## A. GENERAL

- A1. ALL STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE RELEVANT CIVIL, INFRASTRUCTURE, ARCHITECTURAL, MECHANICAL, ELECTRICAL DESIGN DRAWINGS, BOQ AND SPECIFICATIONS. IF ANY DISCREPANCY IS FOUND, THE CONTRACTOR SHALL CONTACT THE ENGINEER IMMEDIATELY BEFORE PROCEEDING WITH THE PORTION OF WORK INVOLVED.
- A2. FOR MAIN SETTING LINES AND LEVELS, REFER TO THE ARCHITECTURAL DRAWINGS. ALL DIMENSIONS MUST BE VERIFIED ON SITE AND ANY DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER FOR CLARIFICATION.
- A3. ALL DIMENSIONS ARE IN MILLIMETER, UNLESS OTHERWISE NOTED, AND ALL LEVELS ARE IN METERS. DO NOT SCALE DRAWINGS READ WRITTEN DIMENSIONS ONLY.
- A4. ALL SECTIONS SHALL BE COORDINATED WITH ARCHITECTURAL (AND OTHER TRADES) DRAWINGS BEFORE ERECTION OF FORMWORK. ANY DISCREPANCY SHALL BE BROUGHT TO THE NOTICE OF THE ENGINEER FOR CLARIFICATION.
- A5. CONSTRUCTION LOADS SHALL NOT EXCEED THE (SIDL+1) KN PER SQUARE METER. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS NOT ATTAINED THE FULL DESIGN STRENGTH.
- A6. REFER TO ARCHITECTURAL (AND OTHER TRADES) DRAWINGS FOR OPENINGS AND SLEEVES IN CONCRETE SLABS NOT SHOWN ON STRUCTURAL DRAWINGS, AND FOR SIZE AND LOCATION OF OPENINGS NOT DIMENSIONED. NO BREAKAGE IS ALLOWED IN CONCRETE AFTER CASTING.
- A7. REFER TO ARCHITECTURAL (AND OTHER TRADE DRAWINGS) FOR POSITIONS AND DIMENSIONS OF OPENINGS IN REINFORCED CONCRETE WALLS AND COORDINATE WITH THOSE SHOWN ON STRUCTURAL DRAWINGS.
- A8. NO OPENINGS OR SLEEVES SHALL BE PLACED IN BEAMS OR COLUMNS EXCEPT AS INDICATED ON STRUCTURAL DRAWINGS AND AS APPROVED.
- A9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER SETTING OUT OF THE WORKS, FOR CORRECTNESS OF LINE AND LEVEL AND FOR QUALITY CONTROL OF THE MATERIALS. THE APPROVAL OF THE ENGINEER SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY AND ANY ERRORS SHALL BE RECTIFIED BY THE CONTRACTOR TO THE APPROVAL OF THE ENGINEER.

## B. DESIGN CRITERIA

### B1. CODES AND STANDARDS

- REINFORCED CONCRETE: BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318M-19
- SEISMIC LOAD DESIGN: INTERNATIONAL BUILDING CODE (IBC 2009) IRAQI SEISMIC CODE
- WIND LOAD DESIGN: AMERICAN SOCIETY OF CIVIL ENGINEERS (ACSE 7-16)

### B2. DESIGN LOADS

#### B2.0 DESIGN CRITERIA

##### 2.1 STANDARDS AND LOADS

- 2.1.1 THE CONCRETE DESIGN IS BASED ON ACI-318-19 AND ALL OTHER RELEVANT AQ CODES.
- 2.1.2 LIVE LOADS ARE BASED ON UNIFORM BUILDING CODE UBC (1997), Imposed Loading Code OR EMPLOYER REQUIREMENTS AND ARE AS FOLLOWS:

STORES AREA	4.80 KN/M <sup>2</sup>
RESTAURANT AREA	3.60 KN/M <sup>2</sup>
ROOF ACCESSIBLE	1.50 KN/M <sup>2</sup>
ROOF NOT ACCESSIBLE	1.00 KN/M <sup>2</sup>
STAIRS	4.80 KN/M <sup>2</sup>

##### 2.1.3 SUPERIMPOSED DEAD LOADS ADOPTED WHERE NOT ACCURATELY COMPUTED:

PARTITION (24CM weight)	VARIES KN/M <sup>2</sup>
TILING	2.00 KN/M <sup>2</sup>
FALSE CEILING, E/M INSTALLATIONS	0.50 KN/M <sup>2</sup>

##### 2.1.4 WIND LOADS (ACSE 7-07) :

SPEED EXPOSURE	100 Km/H B
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## C. REINFORCED CONCRETE CONSTRUCTION

### C1. CONCRETE

#### C1.1 CONCRETE GRADE AND CHARACTERISTICS

COMPRESSIVE STRENGTH OF CONCRETE,  $f_{cu}$ , AS DEFINED BY ASTANDARD 150mm CUBE AT 28 DAYS SHALL BE AS FOLLOWS:

A. CONCRETE IN CONTACT WITH SOIL :

- BLINDING	20 MPa
B. CONCRETE FROM BASEMENT FLOOR TO FIFTH FLOOR (EXCEPT THIRD FLOOR) :	
- COLUMNS AND WALLS	50 MPa
- SUSPENDED BEAMS & SLABS	40 MPa
C. CONCRETE FOR THIRD FLOOR ONLY :	
- COLUMNS AND WALLS	50 MPa
- SUSPENDED BEAMS & SLABS	40 MPa
D. CONCRETE FROM SIXTH FLOOR TO ROOF FLOOR :	
- COLUMNS AND WALLS	50 MPa
- SUSPENDED BEAMS & SLABS	40 MPa

#### GENERAL REQUIREMENTS OF CONCRETE

NO	TESTS	TEST METHOD	SPECIFICATION LIMIT
1.	TEMPERATURE (AT PLACEMENT)	ASTM 1064	32 C° MAX.
2.	SUMP IN mm (AT PLACEMENT)	BS 1881; pet 102	150 ± 25 mm <small>OR AS ADVISED BY THE SUPPLIER</small>
3.	WATER PERMEABILITY	DIN 1048	8mm MAX.

#### C1.2 CONCRETE ADDITIVES

- ALL CONCRETE SHALL CONTAIN AN APPROVED WATER REDUCING, PLASTICIZING ADMIXTURE. HIGH-RANGE, WATER REDUCING ADMIXTURES MAY BE UTILIZED. ALL CONCRETE PERMANENTLY EXPOSED TO THE WEATHER SHALL ALSO CONTAIN AN APPROVED AIR-ENTRAINING ADMIXTURE.
- CONCRETE ADDITIVES USED TO IMPROVE CONSISTENCY, WORKABILITY, QUALITY AND STRENGTH OF CONCRETE SHALL BE APPROVED BY THE ENGINEER.

### C1.3 CONCRETE PROPERTIES

CONCRETE	CEMENT TYPE	MAX. AGGREGATE SIZE (mm)	MIN. CEMENT CONTENT (kg/m <sup>3</sup> )	MAX. W/C RATIO	SGR/ FLY ASH	28 DAYS STRENGTH (MPa)
SUPER STRUCTURE	DPC*	20	400*	0.45*	---	AS PER CL.1
(SUB STRUCTURE)	DPC*	20	400*	0.45*	---	
BLINDING	DPC*	20	250*	0.6*	---	

\* TO BE CONFIRMED BY SOIL SPECIALIST.

### C2. CONCRETE REINFORCEMENT

#### C2.1 GENERAL

- REINFORCEMENT SHALL BE HIGH YIELD (YIELD STRESS = 460MPa ) MARKED "T", THE CONTRACTOR SHALL PROVIDE DETAILED DRAWINGS AND SCHEDULES OF THE REINFORCEMENT REINFORCEMENT FOR THE ENGINEER'S APPROVAL, IN ACCORDANCE
- REINFORCEMENT SHALL CONFORM TO ASTM A615 STANDARDS.
- REINFORCEMENT DETAILS SHOWN ON DRAWINGS ARE INDICATIVE FOR THE PREPARATION OF THE CONTRACTOR'S WORKING DRAWINGS. THE CONTRACTOR SHALL PROVIDE DETAILED SHOP DRAWINGS AND SCHEDULES OF THE REINFORCEMENT FOR THE ENGINEER'S APPROVAL IN ACCORDANCE WITH THE CONTRACT SPECIFICATIONS.
- WHERE BAR LENGTH IS NOT SPECIFIED, LONGEST PRACTICABLE BAR LENGTH SHALL BE EMPLOYED WITH STAGGERED LAP SPLICES. LAP LENGTH SHALL BE A MINIMUM OF 60 TIMES THE BAR DIAMETER, UNLESS OTHERWISE NOTED.
- ALL REINFORCING SPLICES SHALL DEVELOP 100% OF THE TENSILE CAPACITY OF THE REINFORCEMENT. ALTERNATIVE MECHANICAL SPLICES MAY BE CONSIDERED, PROVIDED THAT THEY DEVELOP FULL TENSILE STRENGTH.

#### C2.2 MINIMUM COVER TO REINFORCEMENT

COVER TO ALL REINFORCEMENT SHALL BE AS FOLLOWS UNLESS SHOWN OTHERWISE:	
	= 75mm
	= 75mm EARTH FACES, 40mm OTHER FACES
	= 75mm EARTH FACES, 40mm OTHER FACES
WATER TANK WALLS	= 50mm
SUSPENDED SLABS	= 30mm
BEAMS AND LINTELS	= 40mm
	= 60mm EARTH FACES, 40mm OTHER FACES
	= 30mm WITH THE SPECIFICATIONS.
	= 60mm EARTH FACES, 40mm OTHER FACES

EXTERNAL RELATES TO CONCRETE FACES EXPOSED TO EXTERNAL ENVIRONMENT.

## D. EARTHWORKS, EXCAVATIONS AND DEWATERING

- D1. WHEN EXCAVATING TO FOUNDATION LEVEL CARE SHOULD BE TAKEN NOT TO DISTURB THE UNDERLYING MATERIAL. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED AND ANY SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH GRANULAR FILL COMPACTED BEFORE CONSTRUCTION PROCEEDS. EXCAVATION SHALL AT ALL TIMES BE CARRIED OUT IN DRY CONDITIONS.
- D2. THE CONTRACTOR SHALL TAKE FULL ACCOUNT OF THE SOIL INVESTIGATION INFORMATION AND HIS OWN EXPERIENCE TO DESIGN THE NECESSARY TEMPORARY WORKS. THE DESIGN TOGETHER WITH THE SHOP DRAWINGS SHALL BE SUBMITTED FOR ENGINEER'S APPROVAL PRIOR TO DEWATERING. ALL BACKFILLING SHALL BE COMPACTED TO AT LEAST 95% OF THE MAXIMUM DRY DENSITY AND IN ACCORDANCE WITH THE SPECIFICATIONS.
- D3. IT SHALL REMAIN THE CONTRACTOR'S OBLIGATION TO PROVIDE DRY WORKING CONDITIONS BY ANY MEANS HE MIGHT SEE EFFECTIVE. EXCAVATION SHALL BE AT ALL TIMES CARRIED OUT IN DRY CONDITIONS. PILEGROUND WATER SHALL BE MAINTAINED AT LEAST 500mm BELOW THE DEEPEST POINT IN THE EXCAVATION PILE CAPS WHILE IT REMAINS OPEN. THE DEWATERING SYSTEM USED SHALL NOT AFFECT THE EXISTING ADJACENT FOUNDATION BUILDINGS IN ANY WAY.
- D4. DEWATERING SHALL NOT BE DISCONTINUED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER AND UNTIL AFTER THE CONSTRUCTION OF THE RAFT.

## E. WATERPROOFING

- E1. GENERALLY ALL CONCRETE IN CONTACT WITH SOIL SHALL REQUIRE WATERPROOFING IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE SOILS INVESTIGATIONS REPORT AND AS SHOWN IN THE TYPICAL

## F. BLOCK WALL CONSTRUCTION

- F1. MINIMUM COMPRESSIVE STRENGTH FOR NON-LOAD BEARING HOLLOW BLOCKS SHALL BE 3.5 MPa.
- F2. MINIMUM COMPRESSIVE STRENGTH FOR NON-LOAD BEARING SOLID BLOCKS SHALL BE 7.0 MPa.
- F3. THE CONCRETE HOLLOW BLOCKS SHALL BE FROM AN APPROVED MANUFACTURER WITH APPROPRIATE STRENGTH AND ADEQUATELY CURED AS PER STANDARD SPECIFICATIONS.
- F4. JOINTS BETWEEN CONCRETE BLOCKWORK AND COLUMNS TO BE REINFORCED WITH 200 WIDE GALVANIZED STEEL EXPANDED METAL SECURED BOTH SIDES OF THE JOINT PRIOR TO PLASTERING. CORNER BEADS AND PLASTER STOPS SHALL BE PROVIDED AT ALL CORNERS AND EDGES.
- F5. ALL BLOCK WALLS EXCEEDING 3.0 m IN CLEAR HEIGHT SHALL BE PROVIDED WITH A STIFFENER BEAM AS SHOWN IN THE TYPICAL DETAILS.
- F6. ALL BLOCK WALLS WITH UNBRACED LENGTH EXCEEDING 5.0 m SHALL BE PROVIDED WITH A STIFFENER COLUMN AS SHOWN IN THE TYPICAL DETAILS.

## H. CLADDING SYSTEM DESIGN

- H1. THE CLADDING DESIGN SHALL INCORPORATE SUFFICIENT FLEXIBILITY TO ACCOMMODATE ALL ANTICIPATED MOVEMENTS IN THE STRUCTURE INCLUDING THOSE DUE TO THERMAL EFFECTS, LATERAL MOVEMENTS DUE TO WIND OR SEISMIC LOAD AND AXIAL SHORTENING OF COLUMNS. DETAILS.
- H2. THE CURTAIN WALL DESIGN SHALL CONFORM TO THE SEISMIC DESIGN REQUIREMENTS OF IBC 2009.
- H3. NOT WITHSTANDING THE ABOVE, THE CONTRACTOR SHALL CARRY OUT ANY ADDITIONAL TESTING HE DEEMS NECESSARY TO ENSURE SATISFACTORY PERFORMANCE OF THE CLADDING SYSTEM.

## I. DATUM LEVEL

- I1. ALL LEVELS ARE REFERENCED TO ARCHITECTURAL DATUM LEVEL.

## J. GENERAL FOUNDATIONS NOTES

- J1. ALL FOUNDATIONS SHALL BE CONSTRUCTED UPON WATERPROOFING SYSTEM AND A 100mm CONCRETE BLINDING SLAB WHICH HAS BEEN PLACED OVER AGGREGATE LAYER ACCORDING TO SOIL TEST RECOMMENDATIONS, OVER THE STABILIZED NATURAL SOIL.
- J2. DO NOT BACKFILL AGAINST PIT OR RETAINING WALLS UNTIL THE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH.
- J3. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY WATER, FROM PENETRATING ANY PILED FOUNDATIONS OR STRUCTURAL (HYDROSTATIC) SLABS BEFORE AND AFTER PLACING CONCRETE, AND UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE.
- J4. THE STRUCTURAL CONCRETE FOR EACH PILED FOOTING SHALL BE PLACED IN ONE (1) CONTINUOUS POUR WITH THE STRUCTURAL RAFT. VERTICAL POUR JOINTS SHALL TYPICALLY BE LOCATED IN THE MIDDLE THIRD OF THE SPAN BETWEEN COLUMNS OR WALLS.
- J5. ALL REINFORCING FOR THE FOUNDATIONS, INCLUDING WALL AND COLUMN STARTER BARS SHALL BE PROPERLY SECURED IN PLACE PRIOR TO CONCRETING.
- J6. THE CONTRACTOR SHALL INSTALL THERMOCOUPLE SETS TO MONITOR AND RECORD HEAT GAIN IN THE PILED FOUNDATIONS CONCRETE DURING THE CEMENT HYDRATION PROCESS. ALL THERMOCOUPLE LOCATIONS SHALL BE COORDINATED TO INSURE THAT THE DEVICES ARE NOT DISRUPTED DURING REINFORCING AND CONCRETE PLACEMENT. EACH THERMOCOUPLE SET CONSISTS OF 5 THERMOCOUPLES AND SHALL BE LOCATED IN INDICATED AREAS OF EACH PILED FOUNDATION POUR (IN PLAN) AND POSITIONED AS FOLLOWS:
- ONE (1) THERMOCOUPLE AT THE MID-HEIGHT
- ONE (1) THERMOCOUPLE MIDWAY BETWEEN THE MID-HEIGHT AND THE TOP
- ONE (1) THERMOCOUPLE MIDWAY BETWEEN THE MID-HEIGHT AND THE BOTTOM
- ONE (1) THERMOCOUPLE AT 300MM FROM TOP
- ONE (1) THERMOCOUPLE AT 300MM FROM BOTTOM
- TEMPERATURE READINGS FROM EACH THERMOCOUPLE SHALL BE ELECTRONICALLY RECORDED OVER A 90-DAY PERIOD
- THE CONCRETE MIX DESIGN AND THE CONSTRUCTION TECHNIQUES SHALL BE PREPARED TO LIMIT THE MAXIMUM TEMPERATURE DIFFERENTIAL BETWEEN ANY TWO POINTS WITHIN THE FOUNDATION TO 20 DEGREES CELSIUS, AND A MAXIMUM CONCRETE TEMPERATURE OF 70 DEGREES CELSIUS. THE ADDITION OF ICE AS A REPLACEMENT FOR A PORTION OF MIX WATER, THE REFRIGERATION OF MIX WATER, AND THE COOLING OF AGGREGATES AND CEMENT MAY BE NECESSARY TO REDUCE CONCRETE MIX TEMPERATURE AND HEAT GAIN. THE TEMPERATURE OF THE CONCRETE AT TIME OF PLACEMENT SHALL BE TESTED AND SHALL BE NOT GREATER THAN 32 DEGREES CELSIUS. ANY CONCRETE WITH TEMPERATURE GREATER THAN 32 DEGREES CELSIUS SHALL BE REJECTED.
- J8. CONSISTENCY (SLUMP) TESTS SHALL BE PERFORMED FOR EACH OF THE FIRST 5 TRUCKS SUPPLYING THERMOCOUPLE CONCRETE FOR THE FOUNDATIONS POURS.
- J9. CONCRETE CUBE SAMPLES SHALL BE TAKEN FOR THE FOUNDATIONS. THE REQUIREMENTS ARE AS INDICATED IN THE TECHNICAL SPECIFICATION - "CAST-IN-PLACE CONCRETE".
- J10. REFER TO THE SOIL INVESTIGATION REPORT No. SR 57/2019 DATED ON OCTOBER 2019 FOR ANY ADDITIONAL REQUIREMENTS.

## K. REINFORCED CONCRETE BORED PILES

### K1. GENERAL

- K1. THE SOIL INVESTIGATION REPORT No. SR 57/2023 DATED ON OCTOBER 2023 HAS BEEN PREPARED AND SHALL BE CONSIDERED PART OF THE CONSTRUCTION DOCUMENTATION. THE INFORMATION GIVEN IN THE SOIL REPORT IS SOLELY A GUIDE. RESPONSIBILITY IS ACCEPTED BY THE OWNER OR THE ENGINEER FOR ITS CORRECTNESS.
- K2. ALL PILES SHALL BE BORED CAST IN SITU USING TEMPORARY STEEL CASING.
- K3. ALL REINFORCING BAR SPLICES SHALL DEVELOP 100% OF THE TENSILE CAPACITY OF THE REBAR. ALTERNATIVE MECHANICAL SPLICES MAY BE CONSIDERED, PROVIDED THAT THEY DEVELOP FULL TENSILE STRENGTH.
- K4. ALL PERMANENT PILING CONCRETE SHALL BE DESIGNED FOR A PUMPED TREMIE CONCRETE MIX AND AT LEAST FOR ENHANCED LONG-TERM DURABILITY.
- K5. ALL PILES SHALL UTILIZE SELF-COMPACTING CONCRETE (SCC) AND SHALL BE PLACED IN ONE CONTINUOUS CONCRETE POUR USING THE TREMIE METHOD.
- K6. REINFORCED CONCRETE BORED PILING OF CIRCULAR CONFIGURATION SHALL DEVELOP THE SCHEDULED MINIMUM ALLOWABLE LOAD CAPACITIES WITH A MINIMUM SAFETY FACTOR OF 2.0.
- K7. PILES SHALL BE PLACED A MINIMUM OF 2.5 TIMES THE PILE DIAMETER, CENTER-TO-CENTER OF THE PILES, UNLESS NOTED OTHERWISE.

- K9. THE BOTTOM OF EACH PILE SHALL BE CLEANED OF EXCESS LOOSE MATERIALS BY AIR LIFT PROCEDURES PRIOR TO FINAL CONCRETING.

SHALL BE PLACED TO THE TOP OF THE BORE HOLE, AND THE EXCESS CONCRETE AND LAITANCE MATERIALS TRIMMED BACK AT A LATER DATE.

- K11. THE PILING CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ADDITIONAL LOAD TESTS, MATERIAL TESTING, OR NEW PILES AS THE RESULT OF ANY DEFECTIVE PILES THAT HAVE BEEN INSTALLED OR DEVIATIONS IN PILE LOCATION, VERTICALITY IN EXCESS OF THE ALLOWED TOLERANCES.
- K12. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY LOAD CARRYING CAPACITY OF REINFORCED CONCRETE PILES, PILES ANALYSIS, DESIGN AND FINAL DRAWINGS SHALL BE PREPARED BY PILING SUB CONTRACTOR AND SHALL BE SUBMITTED FOR ENGINEER'S APPROVAL BEFORE CONSTRUCTION.

### K2. PILING MATERIAL

CONCRETE STRENGTH (28 DAY CUBIC STRENGTH)	35 MPa
REINFORCING BARS	EPOXY COATED HIGH TENSILE STEEL (YIELD STRESS = 460MPa)
PILE HEAD GROUT	85 MPa CEMENT GROUT WITH MICROSILICA OR APPROVED EQUIVALENT
CEMENT TYPE	ASTM TYPE-V
MAX. WATER CEMENT RATIO	0.42
MIN CEMENT CONTENT	400 kg/m <sup>3</sup>

### K3. PILE TESTING

- K13. PILES TESTING SHALL CONFORM TO ACI 543 STANDARDS AND PROJECT SPECIFICATION.

- K14. THE CONTRACTOR SHALL ALLOW FOR 2 PRELIMINARY PILES AS LISTED IN THE TABLE BELOW, THE LOCATION OF WHICH SHALL BE AGREED WITH THE ENGINEER. THE PRELIMINARY TEST PILES SHALL BE LOADED UNDER STATIC COMPRESSIVE LOADS TO 1.5 TIMES THE PILE CAPACITY.

PRELIMINARY PILE LOAD TESTS		
PILE DIAMETER (mm)	TEST TYPE	TEST LOAD (kN)
520	COMPRESSION	2200

- K15. THE CONTRACTOR SHALL PERFORM COMPRESSIVE LOAD TESTS ON WORKING PILES AS LISTED IN THE TABLE BELOW. PILES SHALL BE LOADED UNDER STATIC COMPRESSIVE LOADS TO 1.25 TIMES THE PILE CAPACITY.

WORKING PILE LOAD TESTS			
MARK	PILE DIAMETER (mm)	TEST TYPE	TEST LOAD (kN)
P6	520	COMPRESSION	2000
P54	520	COMPRESSION	2000
P62	520	COMPRESSION	2000

- K17. A COMPREHENSIVE INSTRUMENTATION, MONITORING, AND REPORTING PROGRAM FOR THE INSTALLATION OF THE
- K16. STANDARD SONIC INTEGRITY TESTING AT PILE HEAD SHALL BE PERFORMED FOR 100% OF THE NON LOAD TESTED PILES.

TEST PILES , AND THE LOAD TESTING THEREOF SHALL BE ESTABLISHED AND REVIEWED PRIOR TO THE START OF PILING INSTALLATION.

## M. JOINTS

- M.1 THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING PROPOSED CONSTRUCTION / CONTRACTION JOINTS LAYOUT & DETAIL FOR APPROVAL BY THE ENGINEER BEFORE CONSTRUCTION.

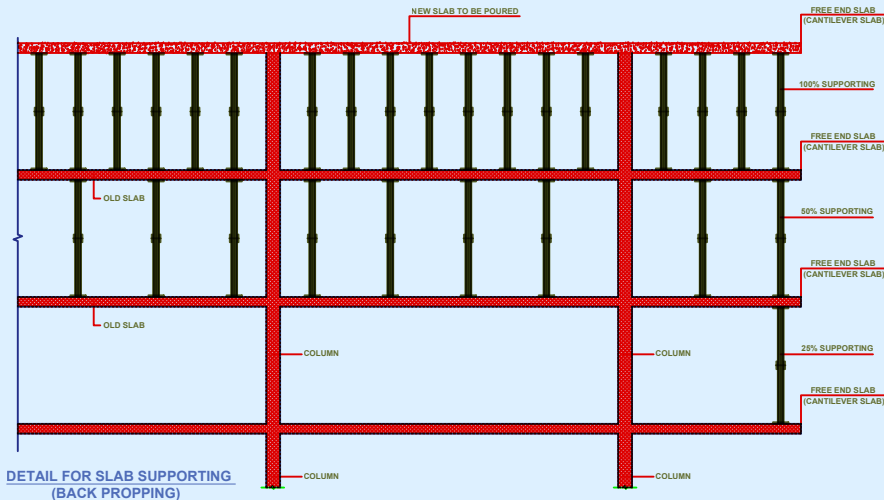
- M.2 CONTRACTION JOINTS SHALL BE IMPLEMENTED AND PERFORMED BY MAX. 10.0m AS PER TYPICAL STRUCTURAL DETAIL.

## N. ABBREVIATIONS

B	BOTTOM BARS
C1	COLUMN NUMBER 1
cm	CENTIMETER
dia	DIAMETER
EJ	EXPANSION JOINT
ES	EACH STEP
EW	EACH WAY
FFL	FINISH FLOOR LEVEL
HOR	HORIZONTAL BARS
L	L SHAPED TOP BARS
LS	HORIZONTAL LINKS AT
Lt	HORIZONTAL LINKS AT SPECIAL
M	MIDDLE BARS
m	METER
MAX	MAXIMUM
MIN	MINIMUM
mm	MILLIMETER
No.	NUMBER
N.T.S	NOT TO SCALE
PC	PLANTED COLUMN
PW	PLANTED WALL
R	REINFORCED STEEL BARS
S	STIRRUPS
SJ	SETTLEMENT JOINT
T	TOP BARS
TOF	TOP LEVEL OF FOUNDATION
TOC	TOP LEVEL OF SLAB
TYP.	TYPICAL
T&B	TOP AND BOTTOM
U	U SHAPED BARS
VER	VERTICAL BARS
VAR.	VARIABLE
W1	SHEAR WALL NUMBER 1
T	HIGH YIELD BARS

## O. ABBREVIATIONS

	STRUCTURAL WALL BELOW SLAB LEVEL
	STRUCTURAL WALL PLANTED ABOVE SLAB LEVEL
	SHOP BEAM
	INVERTED BEAM OR PARAPET ABOVE SLAB LEVEL
	COLUMN BELOW SLAB LEVEL
	COLUMN PLANTED ABOVE SLAB LEVEL
	PLAIN CONCRETE
	SOLID BLOCK WALL
	HOLLOW BLOCK WALL
	SAND FILL
	NATURAL SOIL
	CONCRETE WALL
	WATER PROOFING
	BASECOURSE
	LIGHT WEIGHT FILL
	LIGHT WEIGHT CONCRETE
	PRECAST CONCRETE
	THICKNESS OF SOLID SLABS
	THICKNESS OF RAFT FOUNDATION
	PUNCHING SHEAR REINFORCEMENT DETAIL NO 1
	LEVEL ON PLAN
	LEVEL ON SECTION



no.	date	initials	revision
job title			
(A)			
drawing title			
GENERAL STRUCTURAL NOTES			
designed	ENG - DR.Majid Albana		project manager
checked			scale
			date
drawn			job no.
			sheet no.
approved			

GENERAL :-

1. ALL DIMENSIONS TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND DETAILS. (DO NOT SCALE FROM DRAWINGS).
2. ALL DIMENSIONS ARE IN MILLIMETRES AND ALL LEVELS IN METRES (UNO).
3. THE STRUCTURAL DRAWINGS SHOULD BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL,CIVIL,PLUMBING AND ELECTRICAL DRAWINGS.
4. ALL OPENINGS SIZE AND LOCATION SHOULD BE VERIFIED AND CHECKED WITH SERVICES DRAWINGS,WHERE OPENINGS SIZES ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS, SITE ENGINEER SHALL INTRODUCE SUCH OPENINGS WITH PROPER FRAMING INCLUDING ANY REVISION TO THE SIZES SHOWN ON THE DRAWINGS.
5. DESIGN STANDARD & LOADS :-

• DESIGN & CONSTRUCTION OF REINFORCED CONCRETE STRUCTURES MEMBERS SHALL IN ACCORDANCE WITH ACI-318-95 (ULTIMATE STRENGTH DESIGN METHOD).

• ALL RETANING WALL STRUCTURE SHOULD BE AS BRITISH 8 97- 110 or ACI - 93 - 318.

• MASONARY BRICK OR CONCRETE BLOCK ACCORDING TO B.S - 5628.
6. LOADING :-

• MINIMUM DESIGN LOAD (LIVE LOAD) ACCORDING TO IBC-09.

• SEISMIC LOAD ACCORDING TO IRAQI SEISMIC CODE 1997.

• WIND LOAD ACCORDING TO ASCE-05.
7. FOR TYP. SECTIONS & DETAILS SEE ST-G2.

FOUNDATION AND EARTH WORK :-

1. FOUNDATION DESIGN BASED ACCORDING TO THE SOIL REPORT **PREPARED BY THE & RESEARCH ( )2023\11\ 14).**
2. BEARING CAPACITY ACCORDING TO THE SOIL REPORT IS **(10K/m²)** AT DEPTH OF **(-4.00m)** BELOW THE EXISTING N.G.L.
3. A WELL COMPACTED SUB-BASE LAYERS OF A TOTAL THICK AS INDICATED IN THE DWG. SHOULD BE USED UNDER FOOTING WITH FOLLOWING SPECIFICATIONS :-

• THE DIMENSION OF THE SUB-BASE LAYERS SHOULD BE LARGER THAN THE DIMENSIONS OF THE FOUNDATION FROM ALL SIDES BY 0.25m.

• THE VALUE OF CALIFORNIA BEARING RATIO (C.B.R) SHALL NOT BE LESS THAN (35% ASTM D) 1883 AT 95% OF THE MAXIMUM DRY DENSITY ESTABLISHED ACCORDING TO (ASTM D)1557.

• LIQUID LIMIT ≤ 25%.

• PLASTICITY INDEX ≤ 6%.

• ORGANIC MATERIAL ≤ 2%.

• SO<sub>3</sub> ≤ 5%.

• TOTAL SOLUBLE SALTS ≤ 5%.

• GYPSUM CONTENT ≤ 10.75%.

• RELATIVE COMPACTION 95% (MODIFIED PROCTOR).
4. SULPHATE RESISTANT CEMENT TYPE 5 SHOULD BE USED IN ALL CONCRET WORK IN CONTACT WITH EARTH OR BELOW D.P.C. LEVEL.
5. BACKFILL AROUND FOOTINGS AND UTILITY TRENCH WITHIN THE BUILDING AREA SHOULD BE DONE WITH APPROVED SELECTED CLASSIFIED MATERIAL FREE OF CLAY AND SHOULD BE MECHANICALLY COMPACTED IN LAYERS, NOT EXCEEDING 250mm LOOSE THICKNESS TO 90% OF MAXIMUM PROCTOR DENSITY.

CONSTRUCTION JOINT AND WATERPROOFING :-

1. CONSTRUCTION JOINT :-

• CONSTRUCTION JOINT IN FLOORS SHOULD BE LOCATED WITHIN THE MIDDLE THIRD OF SPANS OF SLABS ,BEAMS & GIRDERS ,JOINT IN GIRDER SHOULD BE OFFSET A MINIMUM DISTANCE OF TWO TIMES THE WIDTH OF INTERSECTING BEAMS.

• AT CONSTRUCTION JOINTS SURFACES SHOULD BE ROUGHENED BY BROOMING OUT MORTAR, EXPOSING 12mm OF COARSE AGGREGATE TWO HOURS AFTER PLACING CONCRETE.

• CONSTRUCTION JOINTS FOR STRUCTURAL SLAB / FOUNDATION / WALLS ETC. AND VOLUME OF CASTING IN A POUR SHOULD BE APPROVED BY THE ENGINEER.

• CONSTRUCTION JOINTS SHOULD BE DOWELED, KEYED AND THOROUGHLY CLEANED, ALL CONSTRUCTION JOINTS SHOULD BE CONSTRUCTED IN ACCORDANCE WITH THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS.CONTRACTOR HAVE TO PREPARE ANY MISSING DETAILS NOT COVERED IN THE STRUCTURAL DRAWINGS AND SUBMIT FOR ENGINEER'S APPROVAL.
2. WATERPROOFING :-

• WATER STOPS SHOULD BE USED AT ALL CONSTRUCTION,CONTRACTION & EXPANSION JOINTS,WHERE WATERPROOFING SYSTEM IS APPLIED ALL INTERSECTION PIECES OF WATER STOPS SHOULD BE FACTORY MOLDED.

• ALL CONCRETE WORKS IN CONTACT WITH SOIL FOR NORMAL STRUCTURE SHOULD BE COATED WITH PROTECTIVE LAYER.
- all dim. from ARCH D.W.G.
- REINFORCED CONCRETE :-
1. COMPRESIVE STRENGTH OF CONCRETE SHOULD BE DETERMIND BY THE TABLE BELOW :-

LOCATIONS MEMBER TYPE	MINIMUM 28 DAYS CUBE COMPRESSIVE STRENGTH(Fcu) ( MPa )	AGGREGATE MAX. SIZE
SCREED	20	10 mm
BLINDING OR LEAN CONCRETE	20	20 mm
SLABS	40	20 mm
PILES	-	20-38 mm
FOUNDATIONS	40	20 mm
COLUMNS AND SHEAR WALLS	50	20 mm
SUSPENDED SLAB, BEAMS AND WALLS	40	20 mm
WATER RETAINING STRUCTURES	-	20 mm
PLAIN CONCRETE	25	20 mm

2. SULPHATE RESISTANT CEMENT TYPE 5 SHOULD BE USED IN ALL CONCRET WORK IN CONTACT WITH EARTH OR BELOW D.P.C LEVEL.

3. REINFORCMENT STEEL CONFORM TO ASTM A615 & A616 OR A617 BARS SHOULD BE GRADE 400 FY=410N/mm (60000psi).

4. PLACING OF REINFORCEMENT SHOULD BE ACCORDING TO ACI-315 DETAILING MANUAL.

5. MINIMUM BARS COVER :-
- | MEMBER                | (mm) |
|-----------------------|------|
| SLABS                 | 25   |
| BEAMS & GIRDERS       | 40   |
| COLUMNS               | 40   |
| INTERIOR WALLS        | 25   |
| EXTERIOR FACE OF WALL | 40   |
| FORMED FOUNDATION     | 50   |
| NON-FORMED FOUNDATION | 75   |
6. MINIMUM BARS SPACING :-

• CLEAR SPACING BETWEEN PARALLEL BARS SHALL NOT BE LESS THAN BAR DIAMETER OR 4/3 OF MAXIMUM AGGREGATE SIZE BUT NOT LESS THAN 25mm.

• CLEAR SPACING BETWEEN LAYERS OF BARS TO BE NOT LESS THAN 25mm AND THE UPPER BARS SHOULD BE OVER THE LOWER BARS .

• IN COLUMNS CLEAR DISTANCE BETWEEN LONGITUDINAL BARS SHOULD BE NOT LESS THAN 1.5 BAR DIAMETER NOR LESS THAN 40mm.

7. MINIMUM LAP LENGTH (UNLESS NOTED ON DRAWINGS) SHOULD BE AS TABLE BELWO :-
- | BAR DIA.(mm)                  | 10  | 12  | 16  | 18  | 20  | 22   | 25   |
|-------------------------------|-----|-----|-----|-----|-----|------|------|
| LAP LENGTH (mm) IN COLUMNS    | 400 | 500 | 600 | 650 | 700 | 800  | 900  |
| LAP LENGTH (mm) IN ELSE WHERE | 400 | 600 | 700 | 800 | 900 | 1000 | 1250 |
- LAP LOCATION IN SLABS AND BEAMS :-

\* AT SUPPORT FOR BOTTOM BARS.

\* AT MID SPAN FOR TOP BARS.

• LAP LOCATION IN FOUNDATION :-

\* AT SUPPORT FOR TOP BARS.

\* AT MID SPAN FOR BOTTOM BARS.

8. VERTICAL REINFORCEMENT IN COLUMN :-

• WHERE COLUMN FACE ARE OFFSET 75mm OR MORE SPLICE OF VERTICAL BARS TO THE OFFSET FACE SHOULD BE MADE BY SEPARATE DOWELS OVER LAP AS SPECIFIED ABOVE.

• WHERE A LONGITUDINAL BARS ARE OFFSET AT SPLICE THE SLOPE OF INCLINED ADJACENT PORTION SHALL NOT EXCEED 1:6 (HORIZONTAL:VERTICAL).

• CHANGING OF REINFORCEMENT BETWEEN FLOORS WHERE SUCH SITUATION OCCURS THE REINFORCEMENT OFF SHOULD BE CUT OFF AT DISTANCE 75mm BELOW FLOOR LEVEL SPACED 100mm AND PLACED BEFORE THE POINT OF BEND.

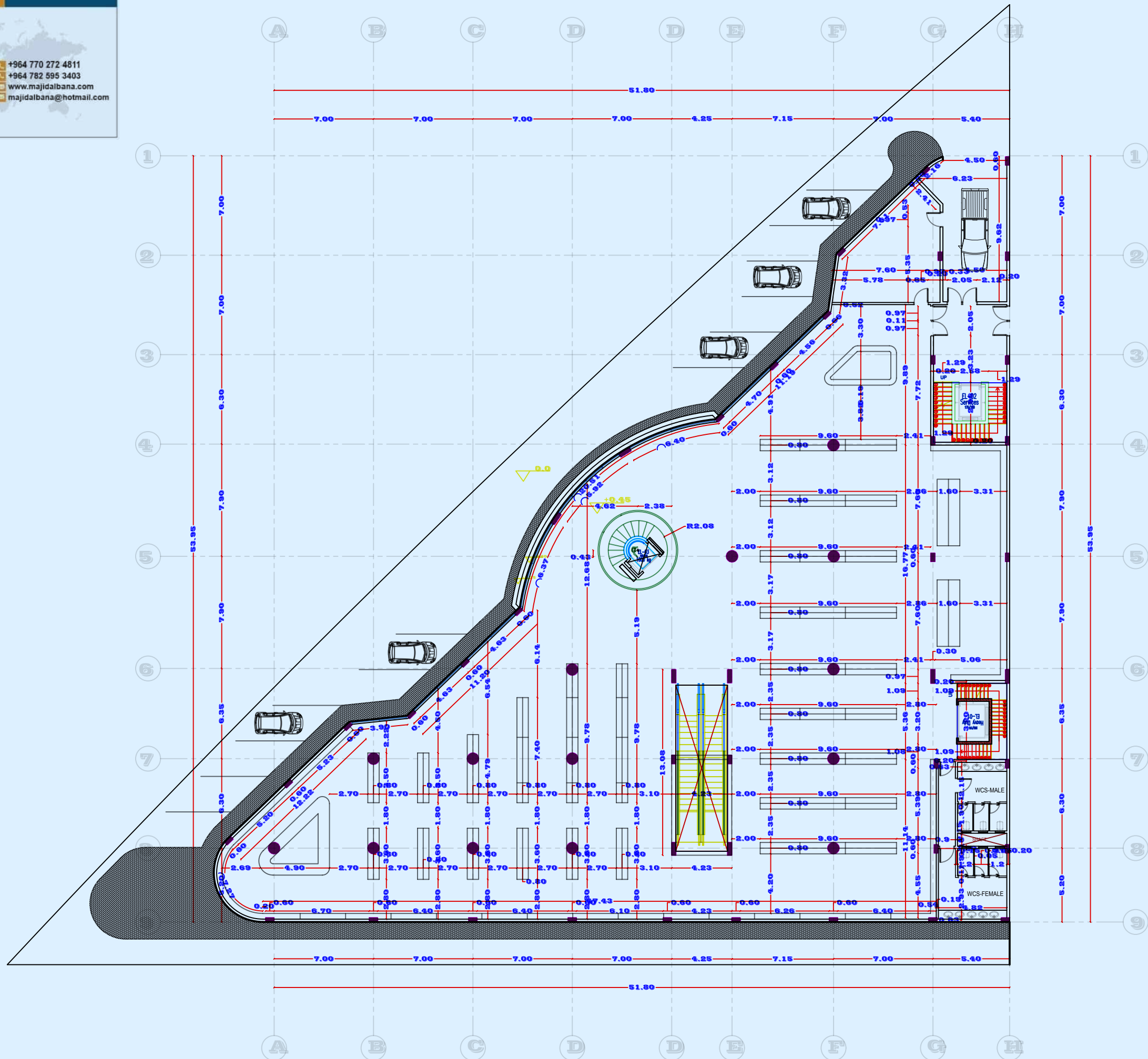
• WHERE LONGITUDINAL BARS OFFSET,PROVIDE 4TIES.

9. HOT & COLD WETHERING SHOULD BE ACCORDING TO ACI-305R-99.

10. ALL REINFORCING BAR BENDS TO BE MADE COLD.

11. IN ONE-WAY SLAB, SHRINKAGE & TEMPERATURE REINF. STEEL EXTENDING IN THE LONG DIRECTION SHALL BE PLACED IN THE PLACE OF, AND TIED TO THE MAIN REINF. EXTENDING IN THE SHORT DIRECTION.

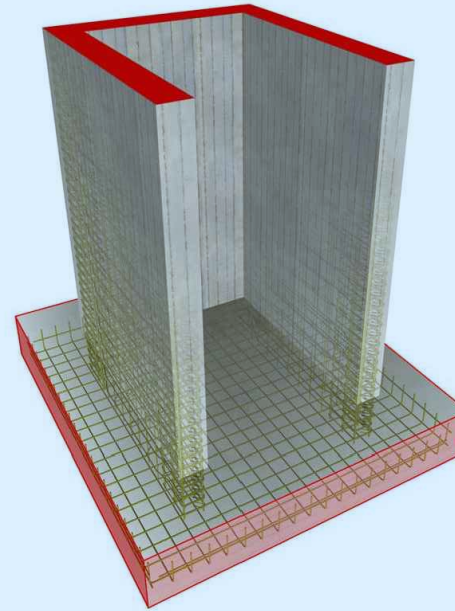
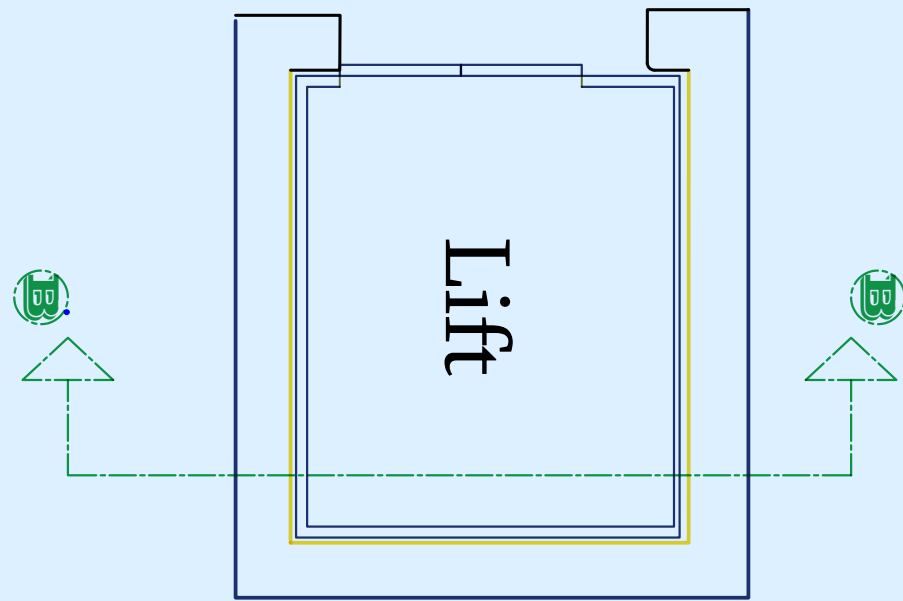
12. MIXING & PLACING CONCRETE SHOULD BE DONE ACCORDING TO ACI - 318M - 95 (CHAPTER 5) CONDUIT OR PIPE SIZE SHALL NOT EXCEED 30% OF SLAB THICKNESS UNLESS SPECIFICALLY DETAILED,OTHERWISE CONCENTRATIONS OF CONDUITS OR PIPES SHOULD BE AVOIDED EXCEPT WHERE DETAILED OPENINGS ARE PROVIDED, ALL SUBJECTED TO ENGINEER'S APPROVAL.
- 
- Typical Detail Of Out Door Yard
- 
- CONSTRUCTION OF PARTITION  
ON GROUND SLAB  
provid construction joint for max.(5mx5m)
- 
- TYPICAL REINF. AROUND  
OPENNINGS UP TO 600
- 
- CONECTION BETWEEN BRICK WALL  
AND R.C. COLUMN  
proposel 1
- 
- Typical Sec. For Stair On Earth
- 
- Typical Sec. Of Ramp
- 
- 
- TYPICAL UP STAND DETAIL  
ROOF OPENNINGS
- 
- CONECTION BETWEEN BRICK WALL  
AND R.C. COLUMN  
proposel 2
- 
- LINTEL REINFORCEMENT
- ABBREVIATIONS :-
- |       |                    |
|-------|--------------------|
| ADD   | ADDITIONAL         |
| ARCH  | ARCHITECTURAL      |
| B     | BEAM               |
| BOTT  | BOTTOM             |
| C1    | COLUMN TYP C1      |
| CANT  | CANTILEVER         |
| CJ    | CONSTRUCTION JOIN  |
| CL    | CENTRE             |
| C     | COULMN             |
| CONC  | CONCRETE           |
| DET   | DETAIL             |
| DIM   | DIMENSION          |
| DWG   | DRAWING            |
| D     | DEPTH              |
| E.A   | EACH               |
| E.F   | EACH FACE          |
| E.J   | EXPANSION JOINT    |
| ELEV  | ELEVATION          |
| E.W   | EACH WAY           |
| EXP   | EXPANSION          |
| F     | FOOTING            |
| F1    | FOOTING TYPE-1     |
| FDN   | FOUNDATION         |
| F.F.L | FINISH FLOOR LEVEL |
| GEN   | GENERAL            |
| GL    | GRID LINE          |
| LL    | LIVE LOAD          |
| MAX   | MAXIMIM            |
| MECH  | MECHANICAL         |
| MIN   | MINIMUM            |
| mm    | MILLIMETRES        |
| SEC   | SECTION            |
- 
- |               |                 |                   |
|---------------|-----------------|-------------------|
|               |                 |                   |
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| no.           | date            | initials revision |
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| job title     |                 |                   |
| (A)           |                 |                   |
| drawing title |                 |                   |
| GENERAL NOTES |                 |                   |
| designed      | project manager |                   |
| checked       | scale           | date              |
| drawn         | job no.         | sheet no.         |
| approved      |                 | 2                 |



no.	date	initials	revision
job title			
(A)			
drawing title			
Plan of Ground floor			
designed	ENG : DR-Majid Albana	project manager	
checked		scale	1-100
drawn		date	10 /2023
approved		job no.	3
		sheet no.	ST/D/06



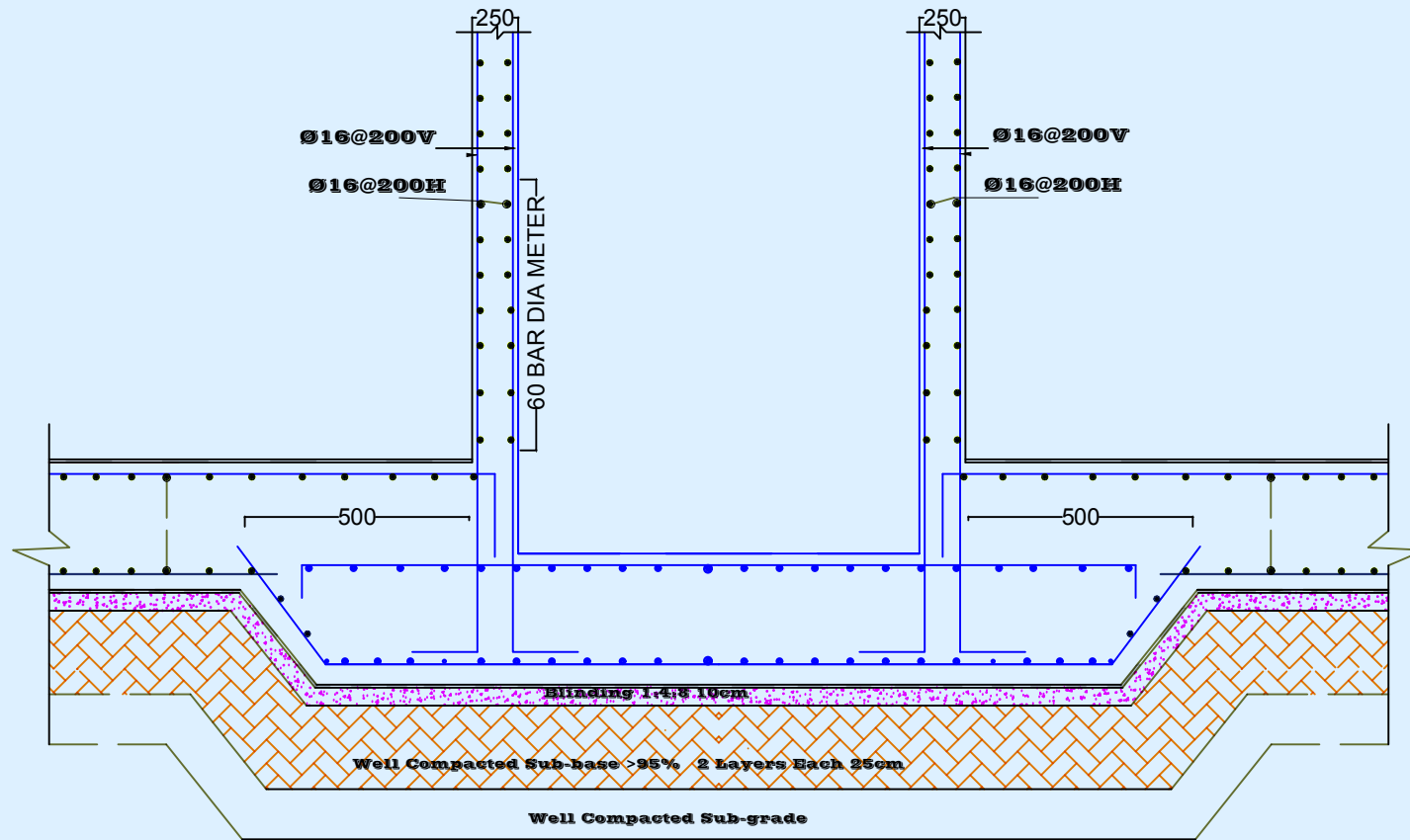




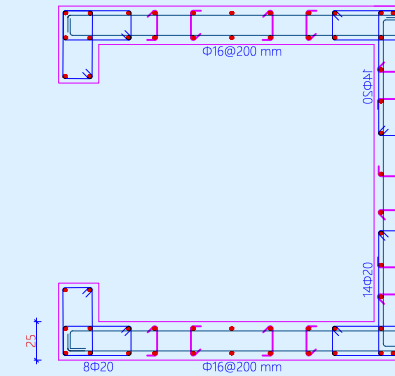


Eng MAJ D A Albana  
المصمم الاستشاري  
د. ماجد البنا

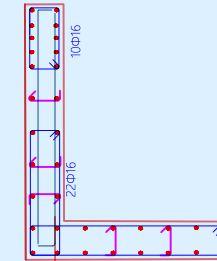
+964 770 272 4811  
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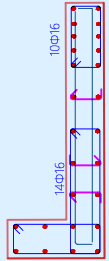
Section B'-B'



Section B-B



W3 Reinf.



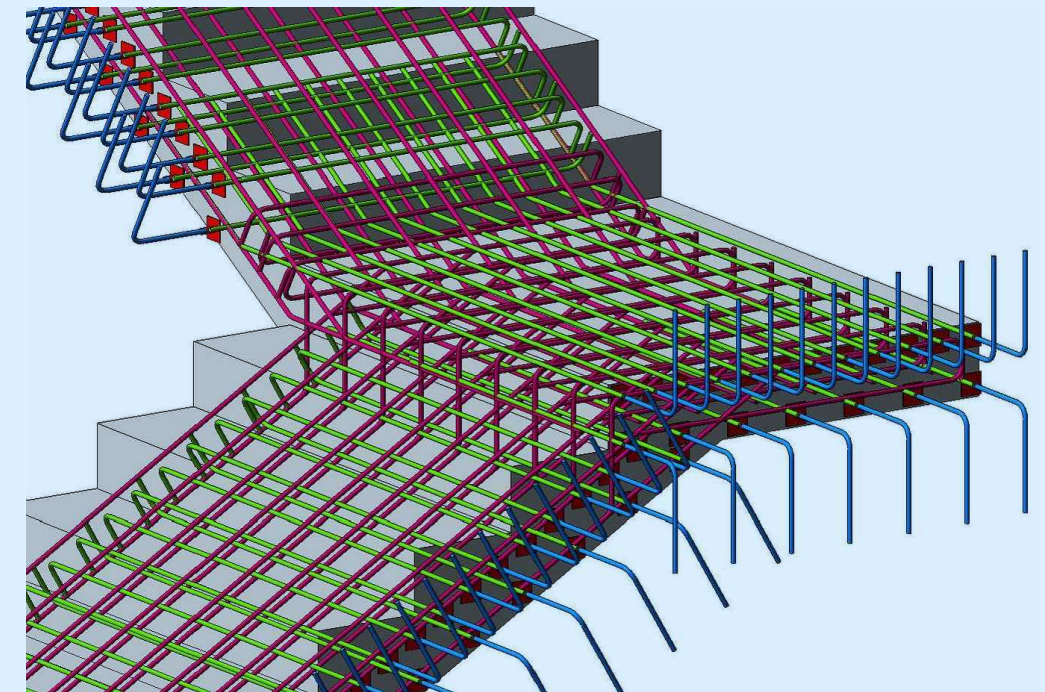
W4 Reinf.

W1 Reinf.

MINIMUM LAP LENGTH (UNLESS NOTED ON DRAWINGS) SHOULD BE AS TABLE BELWO :-

BAR DIA.(mm)	10	12	16	18	20	22	25
LAP LENGTH (mm) IN COLUMNS	400	500	600	650	700	800	900
LAP LENGTH (mm) IN ELSE WHERE	400	600	700	800	900	1000	1250

no.	date	initials	revision
job title			
(A)			
drawing title			
Section A'-A' lift detail			
designed	ENG : Majid Albana	project manager	
checked	ENG : Majid Albana	scale	1-100
drawn	ENG : Majid Albana	job no.	6
approved		date	10 /2023
		sheet no.	ST/D/06



The diagram illustrates the reinforcement layout for a staircase. It shows a series of steps with horizontal and inclined reinforcement bars. The horizontal bars are labeled  $\text{Ø}12@200$ . The inclined bars are labeled  $\text{Ø}16@150$ . The top and bottom horizontal sections are labeled "Slab Reinf.".

Diagram illustrating the reinforcement layout for a staircase slab. The reinforcement consists of horizontal bars (labeled "Slab Reinf.") and diagonal bars (labeled "Ø12@200" and "Ø16@150"). The horizontal bars are spaced at 150 mm (Ø12@150). The diagonal bars are spaced at 200 mm (Ø12@200) and 150 mm (Ø16@150). The reinforcement is shown in a cross-section view of the staircase.

## CONCRETE COVERS

-SLABS	= 25 mm
-BEAMS	= 40 mm
-COLUMNS	= 40 mm
-WALLS	= 25 mm
-SLAB ON GRADE	= 50 mm
-RAFT FOUNDATION	= 75 mm

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no.	date	initials	revision
job title			
(A)			
drawing title			
Stairs detail			
designed ENG : DR-Majid Albana		project manager	
checked	scale 1-100	date 10 / 2023	
drawn	job no.	sheet no.	
approved		7 ST/D/07	

M

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SCHEDULE OF COLUMNS AND WALLS

COLUMNS OR WALLS ID	SIZE		REMARK
	(mm)	WIDTH (mm)	
W1	2000	300	
W2	250		just basement
W3	3000	300	
W4	2500	2500	lift

Notes

-F<sub>cu</sub> = 45 N/mm<sup>2</sup>  
-F<sub>y</sub> = 420 N/mm<sup>2</sup>.

CONCRETE COVERS

-SLABS = 25 mm  
-BEAMS = 40 mm  
-COLUMNS = 40 mm  
-WALLS = 25 mm  
-SLAB ON GRADE = 50 mm  
-RAFT FOUNDATION = 75 mm

-THE GEOTECHNICAL THE BEARING  
CAPACITY OF THE SOIL = 100 kN/m<sup>2</sup>  
-THE BUILDING IS DESIGNED FOR  
BASEMENT + GROUND FLOOR +3 FLOORS  
+ PENT-HOUSE

no.	date	initials	revision		
job title					
(A)					
drawing title					
COLUMN KEY PLAN					
designed	ENG : DR-Majid Albana	project manager			
checked		scale	1-100	date	10/2023
drawn		job no.	8	sheet no.	
approved					ST/D/08

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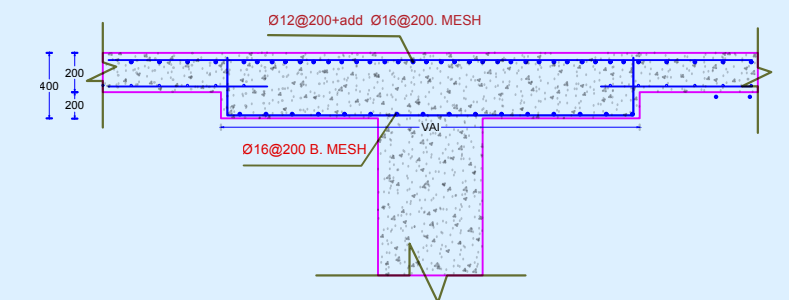
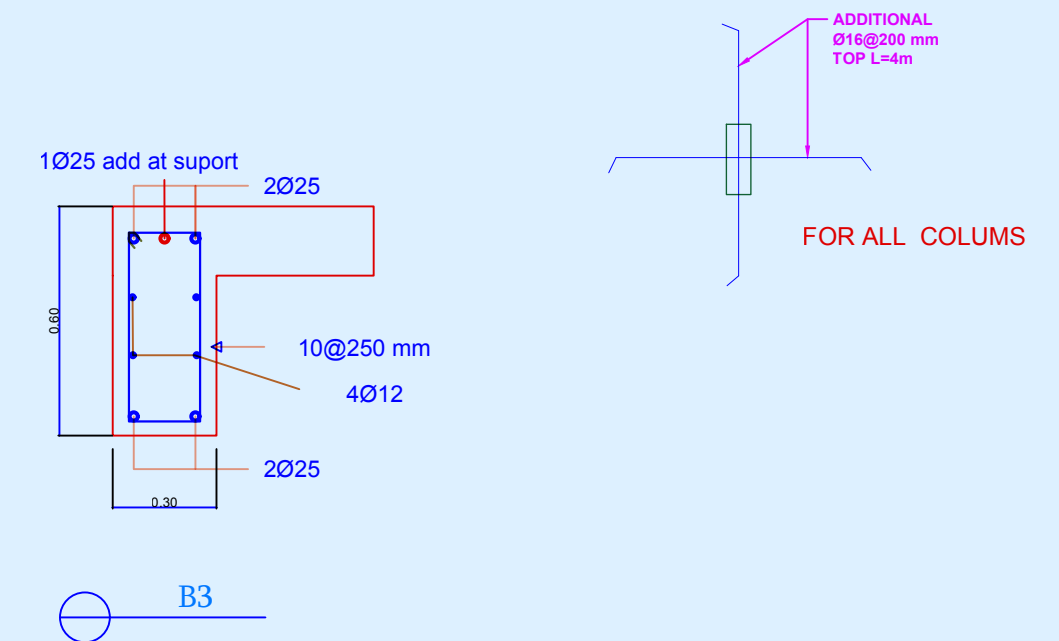
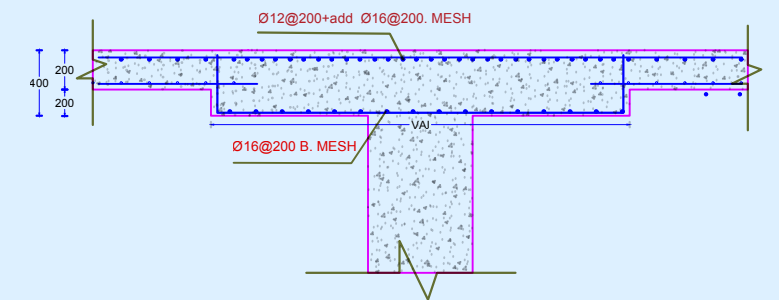
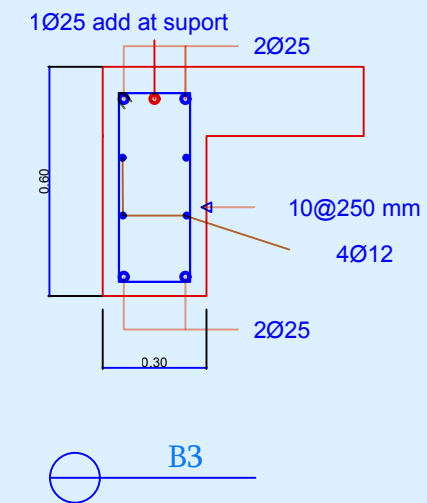


Diagram illustrating the typical slab reinforcement for the ground floor. The slab thickness is 200 mm. The reinforcement includes top reinforcement, bottom reinforcement, and column reinforcement. The diagram shows a cross-section of the slab with reinforcement bars (blue dots) and a grid of reinforcement bars (blue lines) within the columns.

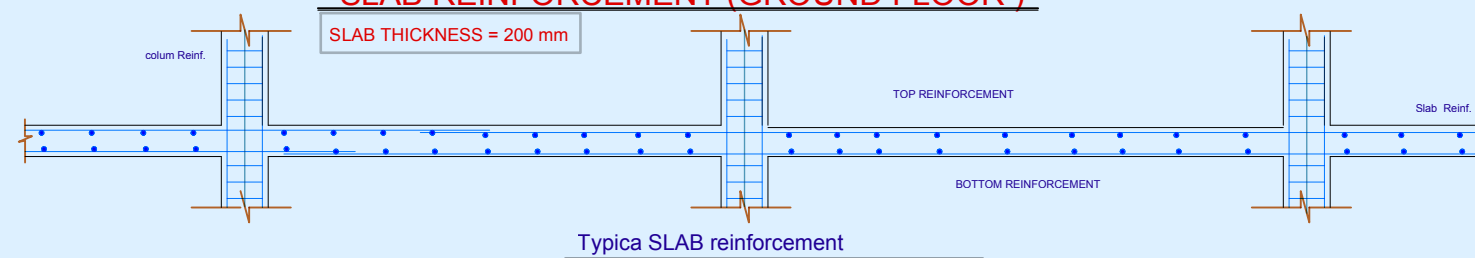
BAR DIA.(mm)	10	12	16	18	20	22	25
LAP LENGTH (mm) IN COLUMNS	400	500	600	650	700	800	900
LAP LENGTH (mm) IN SLAB & BEAMS	400	600	700	800	900	1000	1200

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[illegible]



SLAB THICKNESS = 200 mm

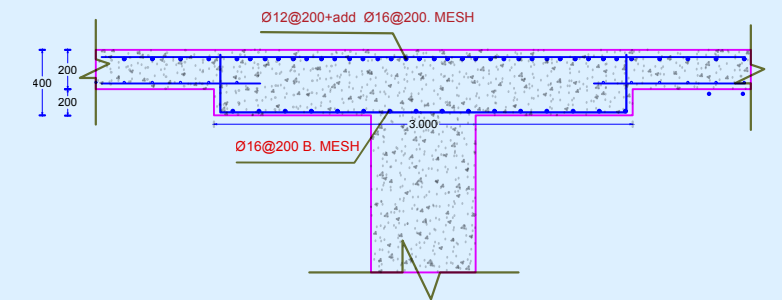
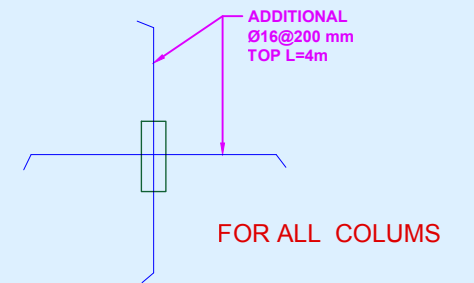
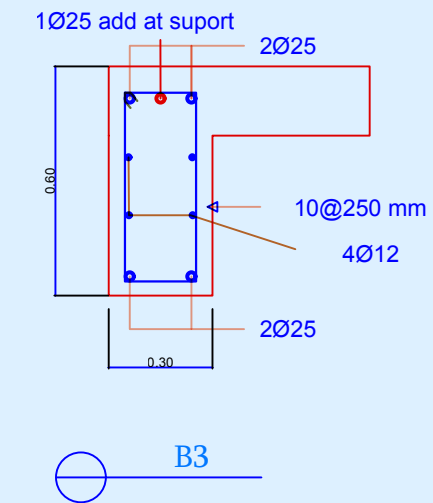


MINIMUM LAP LENGTH (UNLESS NOTED ON DRAWINGS) SHOULD BE AS TABLE BELWO :-

BAR DIA (mm)	10	12	16	18	20	22	25
LAP LENGTH (mm) IN COLUMNS	400	500	600	650	700	800	900
LAP LENGTH (mm) IN SLAB & BEAMS	400	600	700	800	900	1000	1250

. all dim. from ARCH D.W.G.

no.	date	initials	revision
job title			
(A)			
drawing title			
PLAN OF SLAB REINFORCEMENT&SEC.			
designed ENG : DR-Majid Albana		project manager	
checked	scale 1-100	date 10 / 2023	
drawn	job no.	sheet no.	
approved	12		ST/D/11



The diagram illustrates the typical reinforcement layout for a concrete slab. It shows a cross-section of the slab with reinforcement bars (rebar) arranged in a grid pattern. The slab thickness is indicated as 200 mm. The reinforcement is divided into top reinforcement (located near the top surface) and bottom reinforcement (located near the bottom surface). The diagram also shows the reinforcement for the columns (column Reinf.) and the slab (Slab Reinf.).

SLAB THICKNESS = 200 mm

column Reinf.

TOP REINFORCEMENT

Slab Reinf.

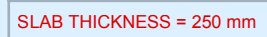
BOTTOM REINFORCEMENT

Typical SLAB reinforcement

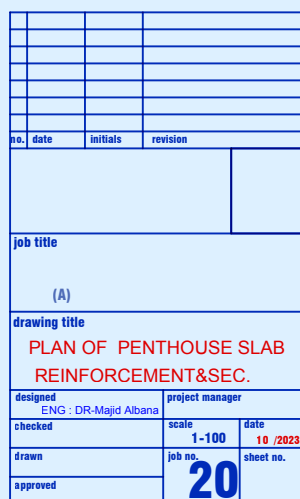
BAR DIA.(mm)	10	12	16	18	20	22	25
LAP LENGTH (mm) IN COLUMNS	400	500	600	650	700	800	900
LAP LENGTH (mm) IN SLAB & BEAMS	400	600	700	800	900	1000	1250

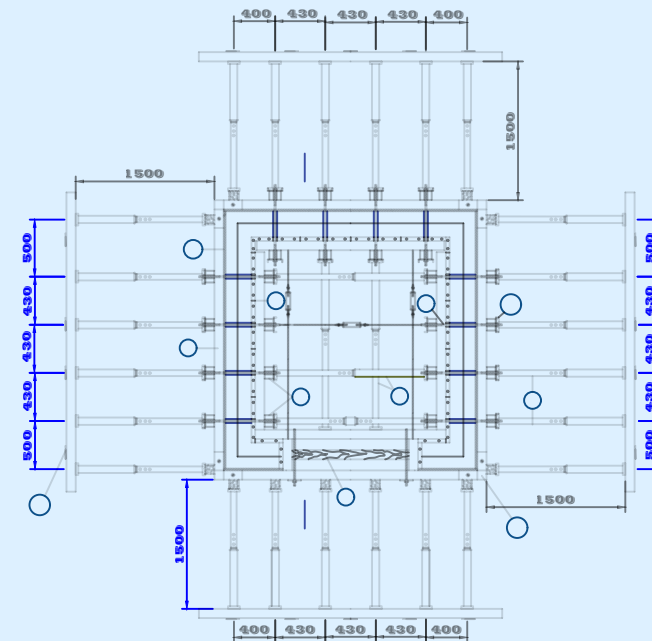
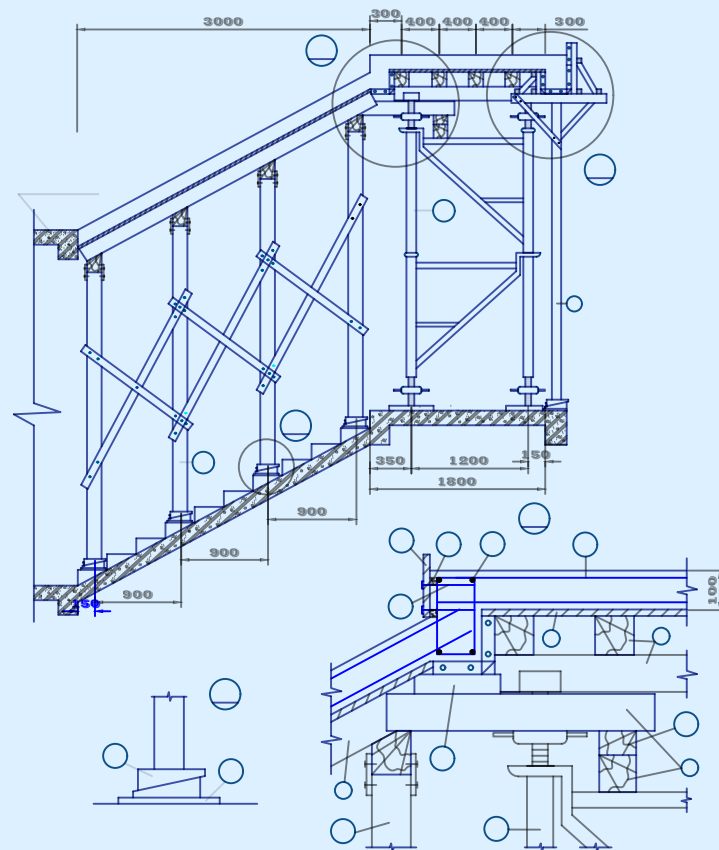
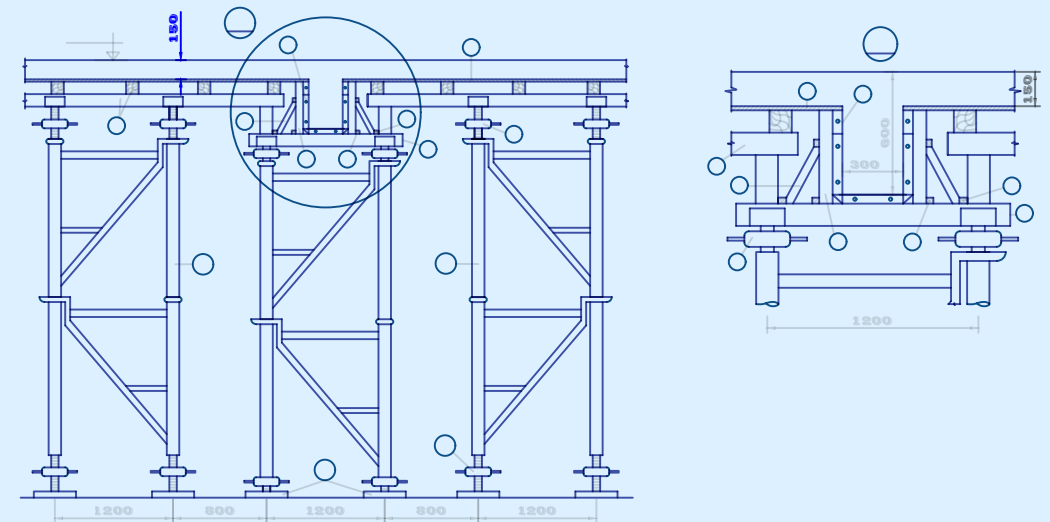
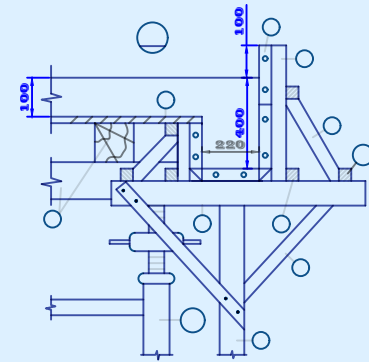
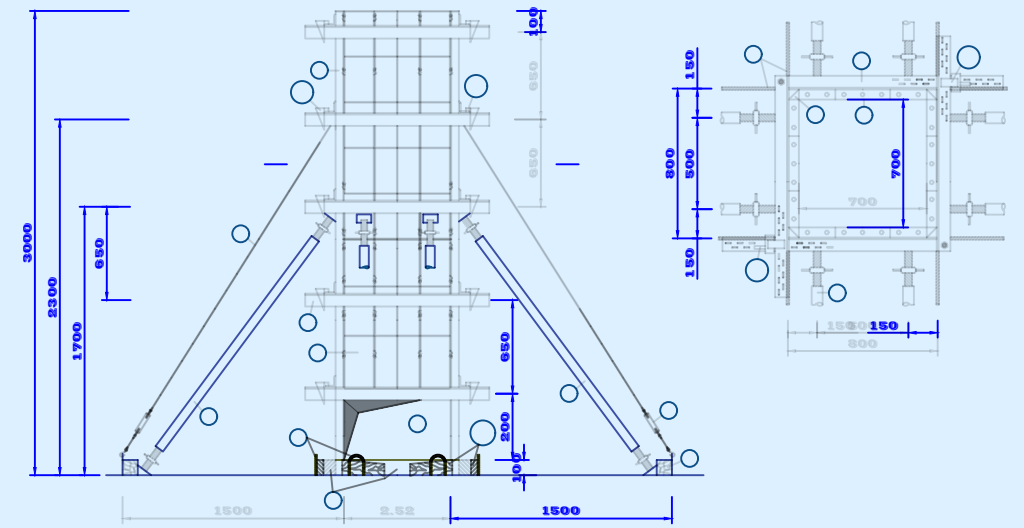
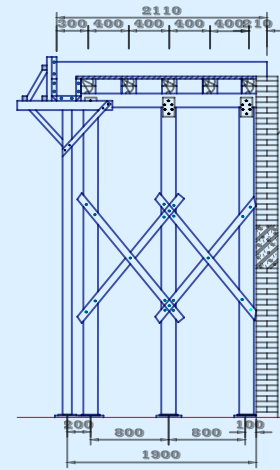
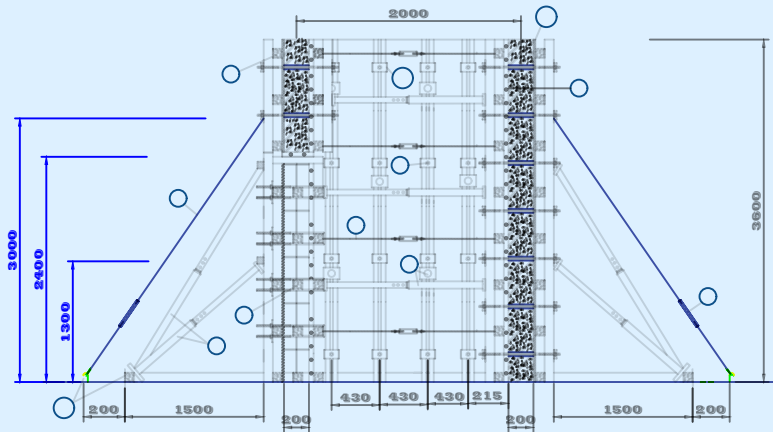
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no.	date	initials	revision
job title			
(A)			
drawing title			
PLAN OF SLAB REINFORCEMENT&SEC.			
designed ENG : DR-Majid Albana		project manager	
checked	scale 1-100	date 10 / 2023	
drawn	job no.	sheet no.	
approved	12 ST/D/11		



no.	date	initials	revision
job title			
(A)			
drawing title			
PLAN OF PENTHOUSE SLAB			
designed ENG : DR-Majid Albana	project manager		
checked	scale 1-100	date 10 /2023	
drawn	job no.	sheet no.	
approved	12		ST/D/11





Composite scaffolding

no.	date	initials	revision
job title			
(A)			
drawing title			
Composite scaffolding			
designed	project manager		
checked	ENG : DR-Majid Albana	scale	date
drawn		1-100	10 /2023
approved		job no.	sheet no.
		17	ST/D/06