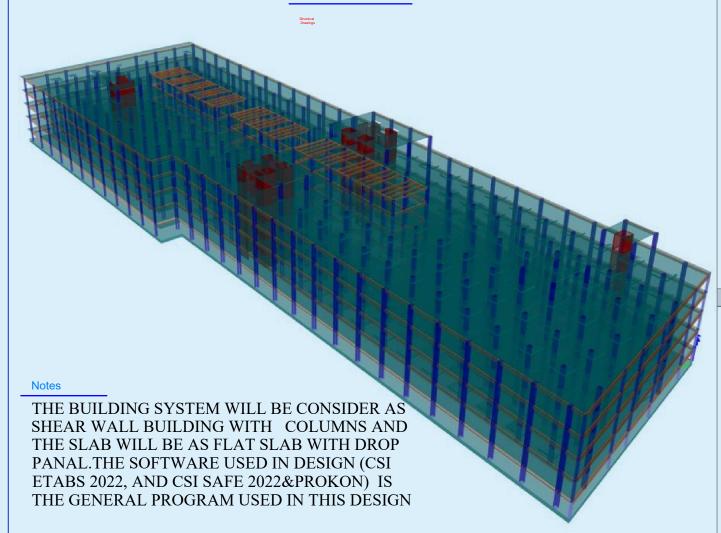
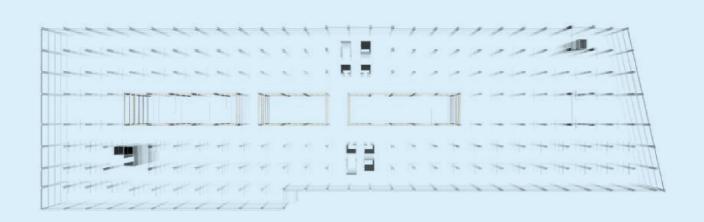
job title

MALL BUILDING









3-D View

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DR-MAJID AL BANA









DESIGNED BY

DR-Majid Albana

CHECKED BY

SCALE

As Shown

DATE

SHEET NO.

Str. 1

A. GENERAL

- AL. ALL STRUCTURAL DRAWINGS SHALL BE READ IN COMMUNICTION WITH THE RELEVANT CIVIL, IMPRASTRUCTURE, ACHITECTURAL, MECHANICAL, ELECTRICAL DESIGN DRAWINGS, BOQ AND SPECIFICATIONS. IF ANY DISCREPANCY IS FOUND, THE CONTRACTOR SHALL CONTACT THE BYIGHTER IMPEDIATELY BEFORE PROCEEDING WITH THE
- A2. FOR MAIN SETTING LINES AND LEVELS, REFER TO THE ARCHITECTURAL DRAWINGS. ALL DIMENSIONS MUST BE VERIFIED ON SITE AND ANY DISCREPANCIES SHALL BE REPORTED TO THE ENGINEER FOR CLARIFICATION.
- A3. ALL DIMENSIONS ARE IN MILLIMETER, UNLESS OTHERWISE NOTED, AND ALL LEVELS ARE IN METERS.

DO NOT SCALE DRAWINGS READ WRITTEN DIMENSIONS ONLY

- A4. ALL SECTIONS SHALL BE COORDINATED WITH ARCHITECTURAL (AND OTHER TRADES) DRAWINGS BEFORE ERECTION OF FORMWORK. ANY DISCREPANCY SHALL BE BROUGHT TO THE NOTICE OF THE ENGINEER FOR CLARIFICATION.
- A5. CONSTRUCTION LOADS SHALL NOT EXCEED THE (SIDL+1) KN PER SQUARE METER. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS NOT ATTAINED THE FULL DESIGN STRENGTH.
- A6. REFER TO ARCHITECTURAL (AND OTHER TRADES) DRAWINGS FOR OPENINGS AND SLEEVES IN CONCRETE SLABS NOT SHOWN ON STRUCTURAL DRAWINGS, AND FOR SIZE AND LOCATION OF OPENINGS NOT DIMENSIONED C2.1 GENERAL NO BREAKAGE IS ALLOWED IN CONCRETE AFTER CASTING.
- A7. REFER TO ARCHITECTURAL (AND OTHER TRADE DRAWINGS) FOR POSITIONS AND DIMENSIONS OF OPENINGS IN REINFORCED CONCRETE WALLS AND COORDINATE WITH THOSE SHOWN ON STRUCTURAL DRAWINGS.
- A8. NO OPENINGS OR SLEEVES SHALL BE PLACED IN BEAMS OR COLUMNS EXCEPT AS INDICATED ON STRUCTURAL DRAWINGS AND AS APPROVED.
- A9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER SETTING OUT OF THE WORKS, FOR CORRECTINES OF LINE AND LEVEL AND FOR QUALITY CONTROL OF THE WARKS, TAX

 ORRECTINESS OF LINE AND LEVEL AND FOR QUALITY CONTROL OF THE MARKSLAS. THE APPROVAL OF THE
 ENGINEER SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY AND ANY ERRORS SHALL
 BE RECTIFIED BY THE CONTRACTOR TO THE APPROVAL OF THE ENGINEER.

 **REINFORCEMENT DETAILS SHOWN ON DRAWINGS ARE INDICATIVE FOR THE PREPARATION OF THE CONTRACTOR
 WORKING DRAWINGS. THE CONTRACTOR SHALL PROVIDE DETAILED SHOP DRAWINGS AND SCHEDULES OF THE
 ENGINEER SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY AND ANY ERRORS SHALL
 BE RECTIFIED BY THE CONTRACTOR TO THE APPROVAL OF THE ENGINEER.

B. DESIGN CRITERIA

B1. CODES AND STAN

- REINFORCED CONCRETE: - SEISMIC LOAD DESIGN:	BUILDING CODE REQUIREMENTS FOR REINFORI INTERNATIONAL BUILDING CODE (IBC 2009)	IRAQI SEISMIC CODE
- WIND LOAD DESIGN:	AMERICAN SOCIETY OF CIVIL ENGINEERS (ACSE 7	7-16)

B2.0 DESIGN CRI

- THE CONCRETE DESIGN IS BASED ON ACI-318-19
 AND ALL OTHER RELEVANT ACI CODES.
- LIVE LOADS ARE BASED ON UNIFORM BUILDING CODE UBC (1997), Iradi Loading Code OR EMPLOYER REQUIREMENTS AND ARE AS FOLLOWS:

STORES AREA	4.80	KN/M2
RESTAURANT AREA	3.60	KNI/M2
ROOF ACCESSIBLE	1.50	KN/M2
ROOF NOT ACCESSIBLE	1.00	KN/M2
CTAIDC	4.00	MI AND

SUPERIMPOSED DEAD LOADS ADOPTED WHERE NOT ACCURATELY CO

2.1.4 WIND LOADS (ASCE 7-07)

C. REINFORCED CONCRETE CONSTRUCTION

C1.1 CONCRETE GRADE AND CHARACTERISTICS

COMPRESSIVE STRENGTH OF CONCRETE, Fcu, AS DEFINED BY ASTANDARD 150mm CUBE AT 28 DAYS SHALL BE AS FOLLOWS: A. CONCRETE IN CONTACT WITH SOIL:

- BLINDING	20.MPa
B. CONCRETE FROM BASEMENT FLOOR TO FIFTH FLOOR (EXCEPT THIRD FLOOR):	45 MPa
- SUSPENDED BEAMS & SLABS	
C. CONCRETE FOR THIRD FLOOR ONLY:	45 MPa
- SUSPENDED BEAMS & SLABS	
D. CONCRETE FROM SIXTH FLOOR TO ROOF FLOOR:	45 MPa
CUCDENIDED DEANS A CLADS	OF MD-

GE	GENERAL REQUIREMENTS OF CONCRETE					
	NO	TESTS	TEST METHOD	SPECIFICATION LIMIT		
	1.	TEMPERATURE (AT PLACEMENT)	ASTM 1064	32 C° MAX.		
	2.	SLUMP IN mm (AT PLACEMENT)	BS 1881; pat 102	150 ± 25 mm OR AS ADVISED BY THE SUPPLIER		
	3.	WATER PERMEABILITY	DIN 1048	8mm MAX.		

ALL CONCRETE SHALL CONTAIN AN APPROVED WATER REDUCING, PLASTICIZING ADMIXTURE. HIGH-RANGE, WATER REDUCING ADMIXTURES MAY BE UTILIZED. ALL CONCRETE PERMANENTLY EXPOSED TO THE WEATHER SHALL ALSO CONTAIN AN APPROVED AIR-ENTRAINING ADMIXTURE.

**ALL CONCRETE SHALL CONTAIN AN APPROVED WATER REDUCING, PLASTICIZING ADMIXTURE. HIGH-RANGE, WATER REDUCING ADMIXTURES MAY BE UTILIZED. ALL CONCRETE PERMANENTLY EXPOSED TO THE WEATHER SHALL ALSO CONTAIN AN APPROVED AIR-ENTRAINING ADMIXTURE.

DATUM LEVEL

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DATUM LEVEL

**TOTAL CONTRACTOR SHALL CARRY OUT ANY ADDITIONAL TESTING HE DESIGN AND APPROVED WATER REDUCING ADMIXTURE. HIGH-RANGE, WATER REDUCIN

CONCRETE	CEMENT TYPE	MAX. AGGREGATE SIZE (mm)	MIN. CEMENT CONTENT (kg/m3)	MAX. W/C RATIO	GGBS/ FLY ASH	28 DAYS STRENGTH (MF
(SUPER STRUCTURE)	OPC*	20	400*	0.45*		
(SUB STRUCTURE)	OPC*	20	400*	0.45*		AS PER C1.1
BLINDING	OPC*	20	250*	0.6*	_	

* TO BE CONFIRMED BY SOIL SPECIALIST.

C2 CONCRETE REINFORCEMENT

- REINFORCEMENT SHALL BE HIGH YIELD (YIELD STRESS = 460MPa) MARKED `T THE CONTRACTOR SHALL PROVIDE DETARMWINGS AND SCHEDULES OF THE REINFORCEMENT REINFORCEMENT FOR THE ENGINEER'S APPROVAL, IN ACCORDANCE

- REINFORCEMENT SHALL CONFORM TO ASTM A615 STANDARDS
- REINFORCEMENT DETAILS SHOWN ON DRAWLINGS ARE INDICATIVE FOR THE PREPARATION OF THE CONTRACTORS THE CONTRACTOR THE INSTALLATION OF THE MIX MINIMUM.

 KIT. A COMPREHENSIVE INSTRUMENTATION, MONITORING, AND REPORTING PROGRAM FOR THE INSTALLATION OF THE MIX MINIMUM.

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 KIT. A COMPREHENSIVE INSTALLATION OF THE MIX MINIMUM.

 KIT. A COMPREHENSIVE
- ALL REINFORCING SPLICES SHALL DEVELOP 100% OF THE TENSILE CAPACITY OF THE REINFORCEMENT.

C2.2 MINIMUM COVER TO REINFORCEMENT

COVER TO ALL REINFORCEMENT SHALL BE AS FOLLOWS UNLESS SHOWN OTHERWISE:

- 75mm
 75mm EARTH FACES, 40mm OTHER FACES
 75mm EARTH FACES, 40mm OTHER FACES
- - = 60mm EARTH FACES, 40mm OTHER FACES
 - = 30mm WITH THE SPECIFICATIONS. = 60mm EARTH FACES, 40mm OTHER FACES

EXTERNAL RELATES TO CONCRETE FACES EXPOSED TO EXTERNAL ENVIRONMENT.

D. EARTHWORKS, EXCAVATIONS AND DEWATERING

- D1. WHEN EXCAVATING TO FOUNDATION LEVEL CARE SHOULD BE TAKEN NOT TO DISTURB THE UNDERLYING WHEN EXAVATION TO COUNDAILION LEVEL CARE SHOULD BE HAREN NOT IN DISTORDING UNDERSON
 MATERIAL. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED AND ANY SOFT SOFTS SHALL BE
 REMOVED AND REPLACED WITH GRANULAR FILL COMPACTED BEFORE CONSTRUCTION PROCEEDS.
 EXCAVATION SHALL AT ALL TIMES BE CARRIED OUT IN DRY CONDITIONS.
- D2. THE CONTRACTOR SHALL TAKE FULL ACCOUNT OF THE SOIL INVESTIGATION INFORMATION AND HIS OWN EXPREIDED TO DESIGN THE MECESSARY TEMPORARY WORKS. THE DESIGN TOGETHER WITH THE SHOP DRAWINGS SHALL BE SUBMITTED FOR ENGINEER'S APPROVAL PRIOR TO DEWATERING, ALL BACKFILLING SHALL BACKFILLING SHALL BE SUBMITTED FOR ENGINEER'S APPROVAL PRIOR TO DEWATERING, ALL BACKFILLING SHALL BACKFILLING SHALL BE COMPACTED TO AT LEAST 95% OF THE MEXIMUM RBY OBISITY AND IN ACCORDANCE

 KH. ALL PERMANENT PILLING CONCRETE SHALL BE DESIGNED FOR A PUMPED TREMIE CONCRETE MIX AND AT LEAST
- PILE SERVICION STATE SHALL BE MAINTAINED AT ALL TIMES CARRIED OUT IN DRY CONDITIONS.

 KS. ALL PILES SHALL UTILIZE SELF-COMPACTING CONCRETE (SCC) AND SHALL BE PLACED IN ONE CONTINUOUS PILE CAPS WILL IT REPAIRS O'RE. THE DEVIATERING SYSTEM USED SHALL NOT AFFECT THE EXISTING ADJACENT COUNTRIBUTIONS IN MAY WENTER. D3. IT SHALL REMAIN THE CONTRACTOR'S OBLIGATION TO PROVIDE DRY WORKING CONDITIONS BY ANY MEANS HE MIGHT SEE EFFECTIVE. EXCAVATION SHALL BE AT ALL TIMES CARRIED OUT IN DRY CONDITIONS.
- D4. DEWATERING SHALL NOT BE DISCONTINUED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER AND UNTIL AFTER THE CONSTRUCTION OF THE RAFT.

E. WATERPROOFING

E1. GENERALLY ALL CONCRETE IN CONTACT WITH SOIL SHALL REQUIRE WATERPROOFING IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE SOILS INVESTIGATIONS REPORT AND AS SHOWN IN THE TYPICAL

F. BLOCK WALL CONSTRUCTION

- FS. ALL BLOCK WALLS EXCEEDING 3.0 m IN CLEAR HEIGHT SHALL BE PROVIDED WITH A STIFFENER BEAM AS SHOWN IN THE TYPICAL DETAILS.

 F6. ALL BLOCK WALLS WITH MERCED LENGTH EXCEEDING 5.0 m SHALL BE PROVIDED WITH A STIFFENER COLUMN AS SHOWN IN THE TYPICAL DETAILS.

H. CLADDING SYSTEM DESIGN

- HI. THE CLADDING DESIGN SHALL INCORPORATE SUFFICIENT FLEXIBILITY TO ACCOMMODATE ALL ANTICIPATED MOVEMENTS IN THE STRUCTURE INCLUDING THOSE DUE TO THERMAL EFFECTS, LATERAL MOVEMENTS DUE TO WIND OR SEISMIC LOAD AND AXIAL SHORTENING OF COLUMNS
- DETAILS.
 H2. THE CURTAIN WALL DESIGN SHALL CONFORM TO THE SEISMIC DESIGN REQUIREMENTS OF IBC 2009.

CONCRETE ADDITIVES USED TO IMPROVE CONSISTENCY, WORKABILITY, QUALITY AND STRENGTH OF CONCRETE
SHALL BE APPROVED BY THE ENGINEER.

1. GENERAL FOUNDATIONS NOTES

- 31. ALL FOUNDATIONS SHALL BE CONSTRUCTED UPON WATERPROOFING SYSTEM AND A 100mm CONCRETE BLINDING SLAB WHICH HAS BEEN PLACED OVER AGGREGATE LAYER ACCORDING TO SOIL TEST RECOMMENDATIONS, OVER THE STABILIZED NATURAL SOIL.
- 32. DO NOT BACKFILL AGAINST PIT OR RETAINING WALLS UNTIL THE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH.
- 13. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY WATER, FROM PENETRATING ANY PILED FOUNDATIONS OR STRUCTURAL (HYDROSTATIC) SLABS BEFORE AND AFTER PLACING CONCRETE, AND UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE.
- 34. THE STRUCTURAL CONCRETE FOR EACH PILED FOOTING SHALL BE PLACED IN ONE (1) CONTINUOUS POUR WITH THE STRUCTURAL RAFT VERTICAL POUR JOINTS SHALL TYPICALLY BE LOCATED IN THE MIDDLE THIRD OF THE SPAN BETWEEN COLUMNS OR WALLS.
- J5. ALL REINFORCING FOR THE FOUNDATIONS, INCLUDING WALL AND COLUMN STARTER BARS SHALL BE PROPERLY SECURED IN PLACE PRIOR TO CONCRETING.
- 36. THE CONTRACTOR SHALL INSTALL THERMOCOUPLE SETS TO MONITOR AND RECORD HEAT GAIN IN THE PILED FOUNDATIONS CONCRETE DURING THE CEMENT HYDRATION PROCESS. ALL THERMOCOUPLE LOCATIONS SHALL BE COORDINATED TO INSURE THAT THE DEVICES ARE NOT DISRUPTED DURING REINFORCING AND CONCRETE PLACEMENT EACH THERMOCOUPLE SET CONSISTS OF STHERMOCOUPLES AND SHALL BE LOCATED IN INDICATED AREAS OF
- EACH THERMOCOUPLE SET CONSISTS OF 5 THERMOCOUPLES AND SHALL BE LOCATED IT EACH PILED FOUNDATION POUR (IN FAM) AND POSITIONED AS FOLLOWS: ONE (1) THERMOCOUPLE AT THE MID-HEIGHT ONE (1) THERMOCOUPLE MIDWAY BETWEEN THE MID-HEIGHT AND THE TOP ONE (1) THERMOCOUPLE MIDWAY BETWEEN THE MID-HEIGHT AND THE BOTTOM ONE (1) THERMOCOUPLE AT 300MM FROM TOP ONE (2) THERMOCOUPLE AT 300MM FROM TOP ONE (2) THERMOCOUPLE AT 300MM FROM TOP

TEMPERATURE READINGS FROM EACH THERMOCOUPLE SHALL BE ELECTRONICALLY RECORDED OVER A 90-DAY PERIOD

SIGN AND THE CONSTRUCTION TECHNIQUES SHALL BE PREPARED TO LINII THE PRAZIMUM RENTIAL BETWEEN ANY TWO POINTS WITHIN THE FORMONATION TO 20 DEGREES CELSIUS, AND TE TEMPERATURE OF 70 DEGREES CELSIUS. THE ADDITION OF ICE AS A REPLACEMENT FOR OF THE NON LOAD TESTED PILES.

OF THE NON LOAD TESTED PILES. THE CONCRETE HIX DESIGN AND THE CONSTRUCTION TECHNIQUES SHALL BE PREPARED TO LIMIT THE MAXIMUM TEMPERATURE DIFFERENTIAL BETWEEN ANY TWO POINTS WITHIN THE FOUNDATION TO 20 DEGREES CESLUS, AND A MAXIMUM CONCRETE TEMPERATURE OF 70 DEGREES CESLUS. THE ADDITION OF ICE AS A REPLACEMENT FOR A PORTION OF MIX WATER, AND ERRIGIERATION OF MIX WATER, AND FOR GREGAECTES AND CEMENT MAY BE NECESSARY TO REDUCE CONCRETE MIX TEMPERATURE AND HEAT GAIN. THE TEMPERATURE OF THE CONCRETE AT THIS OF PLACEMENT SHALL BE TESTED AND SHALL BE NOT GREATER THAN 32 DEGREES CELSIUS. ANY CONCRETE WITH TEMPERATURE GREATER THAN 32 DEGREES CELSIUS SHALL BE REJECTED.

- ALL REINFORCING SPLICES SHALL DEVELOP 100% OF THE TENSILE CAPACITY OF THE REINFORCEMENT.

 38. CONSISTENCY (SLUMP) TESTS SHALL BE PERFORMED FOR EACH OF THE FIRST S TRUCKS SUPPLYING ALTERNATIVE MECHANICAL SPLICES MAY BE CONSIDERED, PROVIDED THAT THEY DEVELOP FULL TENSILE STRENGTH-CONCRETE FOR THE FOUNDATIONS POURS.
 - 39. CONCRETE CUBE SAMPLES SHALL BE TAKEN FOR THE FOUNDATIONS. THE REQUIREMENTS ARE AS INDICATED IN THE TECHNICAL SPECIFICATION - "CAST-IN-PLACE CONCRETE"
 - 310. REFER TO THE SOIL INVESTIGATION REPORT No. SR 57/2019 DATED ON OCTOBER 2019
 - FOR ANY ADDITIONAL REQUIREMENTS.

K. REINFORCED CONCRETE BORED PILES

- K1. GENERAL
- K1. THE SOIL INVESTIGATION REPORT NO. SR 57/2023 DATED ON OCTOBER 2023HAS BEEN PREPARED AND SHALL BE CONSIDERED PART OF THE CONSTRUCTION DOCUMENTATION. THE INFORMATION GIVEN IN THE SOIL REPORT IS SOLELY A GUIDE. RESPONSIBILITY IS ACCEPTED BY THE OWNER OR THE ENGINEER FOR TIS CORRECTIVES.
- K2. ALL PILES SHALL BE BORED CAST IN SITU USING TEMPORARY STEEL CASING.
- K4. ALL PERMANENT PILING CONCRETE SHALL BE DESIGNED FOR A PUMPED TREMIE CONCRETE MIX AND AT LEAST

- K6. REINFORCED CONCRETE BORED PILING OF CIRCULAR CONFIGURATION SHALL DEVELOP THE SCHEDULED MINIMUM ALLOWABLE LOAD CAPACITIES WITH A MINIMUM SAFETY FACTOR OF 2.0.
- K7. PILES SHALL BE PLACED A MINIMUM OF 2.5 TIMES THE PILE DIAMETER, CENTER-TO-CENTER OF THE PILES, UNLESS NOTED OTHERWISE.
- K9. THE BOTTOM OF EACH PILE SHALL BE CLEANED OF EXCESS LOOSE MATERIALS BY AIR LIFT PROCEDURES PRIOR TO FINAL CONCRETING.
- SHALL BE PLACED TO THE TOP OF THE BORE HOLE, AND THE EXCESS CONCRETE AND LAITANCE MATERIALS
- FI. MINIMUM COMPRESSIVE STRENGTH FOR NON-LOAD BEARING HOLLOW BLOCKS SHALL BE 3.5 MPa.
 F2. MINIMUM COMPRESSIVE STRENGTH FOR NON-LOAD BEARING SOLID BLOCKS SHALL BE 7.0 MPa.
 F3. THE CONCRETE HOLLOW BLOCKS SHALL BE FROM AN APPROVED MANUFACTURER WITH APPROPRIATE
 STRENGTH AND AREQUIATEV CURED AS FPE STANDARD SPECIFICATIONS.
 F4. JOINTS BETWEEN CONCRETE BLOCKWORK AND COLUMNS TO BE REINFORCED WITH 200 WIDE GALVANIZED
 STEEL EXPANDED METAL SECURE BOTH SIDES OF THE JOINT FROM TO PLASTERING. CORNER BEADS AND
 PLASTER STOPS SHALL BE PROVIDED AT ALL CORNERS AND EDGES.

 K11. THE PILING CONTRACTOR SHALL BE RESURT OF ALL ADDITIONAL LOAD TESTS, MATERIAL TESTING,
 OR NEW PLESS AS THE RESULT OF ANY DEFECTIVE PILES THAT HAVE BEEN INSTALLED ON DEVIATIONS IN
 PILE CONTRACTOR SHALL BE RESULT OF ANY DEFECTIVE PILES THAT HAVE BEEN INSTALLED ON DEVIATIONS IN
 PILE CONTRACTOR SHALL BE RESULT OF ANY DEFECTIVE PILES THAT HAVE BEEN INSTALLED ON DEVIATIONS IN
 PILE CONTRACTOR SHALL BE PROVIDED AT ALL CORNERS AND EDGES.

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 OR NEW PLESS AS THE RESULT OF ANY DEFECTIVE PILES THAT HAVE BEEN INSTALLED ON DEVIATIONS IN
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 PILE CONTRACTOR SHALL BE RESULT OF ANY DEFECTIVE PILES THAT HAVE BEEN INSTALLED ON DEVIATIONS IN
 PILE CONTRACTOR SHALL BE RESULT OF ANY DEFECTIVE PILES THAT HAVE BEEN INSTALLED ON DEVIATIONS IN
 PILE CONTRACTOR SHALL BE RESULT OF ANY DEVELOPMENT IN PROPERTY.

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 FIRST THE CONTRACTOR SHALL BE RESULT OF ALL DOTTION.

 FIRST THAT HAVE CONTRACTOR SHALL BE R
 - CONCRETE PILES, PILES ANALYSIS, DESIGN AND FINAL DRAWINGS SHALL BE PREPARED BY PILING SUB CONTRACTOR AND SHALL BE SUBMITTED FOR ENGINNER'S APPROVAL BEFORE CONSTRUCTION.

K2. PILING MATERIAL

CONCRETE STRENGTH (28 DAY CUBIC STRENGTH)	35 MPa	
REINFORCING BARS	EPOXY COATED HIGH TENSILE STEEL (YIELD STRESS = $460MPa$)	
PILE HEAD GROUT	85 MPa CEMENT GROUT WITH MICROSILICA OR APPROVED EQUIVALENT	
CEMENT TYPE	ASTM TYPE-V	
MAX. WATER CEMENT RATIO	0.42	
MIN CEMENT CONTENT	400 kg/m3	

K3 PILE TESTING

- K13. PILES TESTING SHALL CONFORM TO ACI 543 STANDARDS AND PROJECT SPECIFICATION.
- K14. THE CONTRACTOR SHALL ALLOW FOR 2 PRELIMINARY PILES AS LISTED IN THE TABLE BELOW, THE LOCATION OF WHICH SHALL BE AGREED WITH THE ENGINEER. THE PRELIMINARY TEST PILES SHALL BE LOADED UNDER STATIC COMPRESSIVE LOADS TO 1.5 TIMES THE PILE CAPACITY.

ĺ	PRELIN	INARY PILE LOA	AD TESTS
	PILE DIAMETER (mm)	TEST TYPE	TEST LOAD (kN)
ı	520	COMPRESSION	2200

K15. THE CONTRACTOR SHALL PERFORM COMPRESSIVE LOAD TESTS ON WORKING PILES AS LISTED IN THE TABLE BELOW, PILES SHALL BE LOADED UNDER STATIC COMPRESSIVE LOADS TO 1,25 TIMES THE PILE CAPACITY.

WORKING PILE LOAD TESTS				
MARK	PILE DIAMETER (mm)	TEST TYPE	TEST LOAD (kN)	
P6	520	COMPRESSION	2000	
P54	520	COMPRESSION	2000	
P62	520	COMPRESSION	2000	

TEST PILES , AND THE LOAD TESTING THEREOF SHALL BE ESTABLISHED AND REVIEWED PRIOR TO THE START

M. JOINTS

B BOTTOM BARS C1 COLUMN NUMBER 1

PRELIN	IMINARY PILE LOAD TESTS		
PILE DIAMETER (mm)	TEST TYPE	TEST LOAD (kN)	
520	COMPRESSION	2200	

cn	1 CENTIMETER	+ +	DROP BEAM
Di	a DIAMETER		INVERTED BEAM OR PARAPET ABOV
EJ	EXPANSION JOINT		COLUMN BELOW SLAB LEVEL
ES	EACH STEP		COLUMN PLANTED ABOVE SLAB LEV
E١	V EACH WAY	10 16 20 20 20 20	PLAIN CONCRETE
FF	L FINISH FLOOR LEVEL		SOLID BLOCK WALL
Н	OR HORIZONTAL BARS		HOLLOW BLOCK WALL
L	L SHAPED TOP BARS	Bunh Coulom	SAND FILL
Ls	HORIZONTAL LINKS AT		NATURAL SOIL
	CONCRETE WALL	200,000,000,000	COMPACTED FILL
Lt	HORIZONTAL LINKS AT SPEC	CIAL	WATER PROOFING
	BOUNDARY ELEMENT	Mary Charles	BASECOURSE
М	MIDDLE BARS	A	LIGHT WEIGHT FILL
m	METER	Employed Andrews	LIGHT WEIGHT CONCRETE
M	AX MAXIMUM		PRECAST CONCRETE

- - N.T.S NOT TO SCALE
 - PLANTED COLUMN
 PLANTED WALL
 MILD STEEL BARS
 STIRRUPS
 SETTLEMENT JOINT
 - TOP BARS TOF TOP LEVEL OF FOUNDATION TOC TOP LEVEL OF SLAB
 - YP. TYPICAL '&B TOP AND BOTTOM U U SHAPED BARS VER VERTICAL BARS VAR VARIABLE

SHEAR WALL NUMBER 1 HIGH YIELD BARS

المصمم الاستشارات M.1 THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING PR CONSTRUCTION / CONTRACTION JOINTS LAYOUT & DETAIL FOR / PPROVAL BY THE ENGINEER BEFORE CONSTRUCTION.

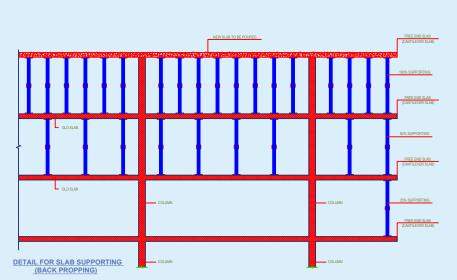
M.2 CONTRACTION JOINTS SHALL BE IMPLEMENTED AND PERFORME MAX. 10.0m AS PER TYPICAL STRUCTURAL DETAIL.

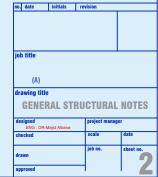
















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GENERAL:-

- 1. ALL DIMENSIONS TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS AND
- 2. ALL DIMENSIONS ARE IN MILLIMETRES AND ALL LEVELS IN METRES (UNO).
- THE STRUCTURAL DRAWINGS SHOULD BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, CIVIL, PLUMBING AND ELECTRICAL DRAWINGS.
- 4. ALL OPENINGS SIZE AND LOCATION SHOULD BE VERIFIED AND CHECKED WITH SERVICES DRAWINGS, WHERE OPENINGS SIZES ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS, SITE ENGINEER SHALL INTRODUCE SUCH OPENINGS WITH PROPER FRAMING INCLUDING ANY REVISION TO THE SIZES SHOWN ON THE DRAWINGS.
- 5. DESIGN STANDARED & LOADS :-
- DESIGN & CONSTRUCTION OF REINFORCED CONCRETE STRUCTURES MEMBERS SHALL IN ACCORDANCE WITH ACI-318-95 (ULTIMATE STRENGTH DESIGN METHOD).
- ALL RETANING WALL STRUCTURE SHOULD BE AS BRITISH 8 97- 110 or ACI 93 318.
- MASONARY BRICK OR CONCRETE BLOCK ACCORDING TO B.S 5628.
- 6 LOADING :-
- MINIMUM DESIGN LOAD (LIVE LOAD) ACCORDING TO IBC-09.
- SEISMIC LOAD ACCORDING TO IRAQI SEISMIC CODE 1997.
- WIND LOAD ACCORDING TO ASCE-05.
- 7. FOR TYP. SECTIONS & DETAILS SEE ST-G2.

FOUNDATION AND EARTH WORK:-

- 1. FOUNDATION DESIGN BASED ACCORDING TO THE SOIL REPORT PREPARED BY THE
- 2. BEARING CAPACITY ACCORDING TO THE SOIL REPORT IS (10K/m²) AT DEPTH OF (-4,00m) BELOW THE EXISTING N.G.L.
- 3. A WELL COMPACTED SUB-BASE LAYERS OF A TOTAL THICK AS INDICATED IN THE DWG. SHOULD BE USED UNDER FOOTING WITH FOLLOWING SPECIFICATIONS:-
- THE DIMENSION OF THE SUB-BASE LAYERS SHOULD BE LARGER THAN THE DIMENSIONS OF THE FOUNDATION FROM ALL SIDES BY 0.25m.
- THE VALUE OF CALIFORNIA BEARING RATIO (C.B.R) SHALL NOT BE LESS THAN (35% ASTM D) 1883 AT 95% OF THE MAXIMUM DRY DENSITY ESTABLISHED ACCORDING TO (ASTM D)1557.
- LIQUID LIMIT ≤ 25%
- PLASTICITY INDEX ≤ 6%
- ORGANIC MATERIAL ≤ 2%.
- $SO_2 \le 5\%$.
- TOTAL SOLUBLE SALTS ≤ 5%.
- GYPSUM CONTENT ≤ 10.75%.
- RELATIVE COMPACTION 95% (MODIFIED PROCTOR).
- 4. SULPHATE RESISTANT CEMENT TYPE 5 SHOULD BE USED IN ALL CONCERET WORK IN CONTACT WITH EARTH OR BELOW D.P.C. LEVEL
- 5. BACKFILL AROUND FOOTINGS AND UTILITY TRENCH WITHIN THE BUILDING AREA SHOULD BE DONE WITH APPROVED SELECTED CLASSIFIED MATERIAL FREE OF CLAY AND SHOULD BE MECHANICALLY COMPACTED IN LAYERS, NOT EXCEEDING 250mm LOOSE THICKNESS TO 90% OF MAXIMUM PROCTOR DENSITY.

CONSTRUCTION JOINT AND WATERPROOFING:-

- 1. CONSTRUCTION JOINT :-
- CONSTRUCTION JOINT IN FLOORS SHOULD BE LOCATED WITHIN THE MIDDLE THIRD OF SPANS OF SLABS ,BEAMS & GIRDERS,JOINT IN GIRDER SHOULD BE OFFSET A MINIMUM DISTANCE OF TWO TIMES THE WIDTH OF INTERSECTING BEAMS.
- AT CONSTRUCTION JOINTS SURFACES SHOULD BE ROUGHENED BY BROOMING OUT. MORTAR, EXPOSING 12mm OF COARSE AGGREGATE TWO HOURS AFTER PLACING
- CONSTRUCTION JOINTS FOR STRUCTURAL SLAB / FOUNDATION / WALLS ETC. AND VOLUME OF CASTING IN A POUR SHOULD BE APPROVED BY THE ENGINEER.
- CONSTRUCTION JOINTS SHOULD BE DOWELED. KEYED AND THOROUGHLY CLEANED ALL CONSTRUCTION JOINTS SHOULD BE CONSTRUCTED IN ACCORDANCE WITH THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS CONTRACTOR HAVE TO PREPARE ANY MISSING DETAILS NOT COVERED IN THE STRUCTURAL DRAWINGS AND SUBMIT FOR ENGINEER'S APPROVAL.
- 2. WATERPROOFING :-
- WATER STOPS SHOULD BE USED AT ALL CONSTRUCTION, CONTRACTION & EXPANSION JOINTS, WHERE WATERPROOFING SYSTEM IS APPLIED ALL INTERSECTION PIECES OF WATER STOPS SHOULD BE FACTORY MOLDED.
- ALL CONCRETE WORKS IN CONTACT WITH SOIL FOR NORMAL STRUCTURE SHOULD BE
- all dim. from ARCH D.W.G.

REINFORCED CONCRETE:-

1. COMPRESIVE STRENGTH OF CONCRETE SHOULD BE DETERMIND BY THE TABLE BELOW :-

LOCATIONS MEMBER TYPE	MINIMUM 28 DAYS CUBE COMPRESSIV STRENGTH(Fcu) (MPa)	E AGGREGATE MAX. SIZE
SCREED	20	10 mm
BLINDING OR LEAN CONCRE	TE 20	20 mm
SLABS	35	20 mm
PILES	-	20-38 mm
FOUNDATIONS	40	20 mm
COLUMNS AND SHEAR WALL	S 45	20 mm
SUSPENDED SLAB, BEAMS AND WALLS	35	20 mm
WATER RETAINING STRUCTU	RES _	20 mm
PLAIN CONCRETE	25	20 mm

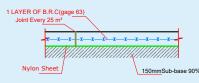
- 2. SULPHATE RESISTANT CEMENT TYPE 5 SHOULD BE USED IN ALL CONCERET WORK IN CONTACT WITH EARTH OR BELOW D.P.C LEVEL.
- 3. REINFORCMENT STEEL CONFORM TO ASTM A615 & A616 OR A617 BARS SHOULD BE GRADE 400 FY=410N/mm (60000psi)
- 4. PLACING OF REINFORCEMENT SHOULD BE ACCORDING TO ACI-315 DETAILING MANUAL.
- 5. MINIMUM BARS COVER :-

MEMBER	(mm)
SLABS	25
BEAMS & GIRDERS	40
COLUMNS	40
INTERIOR WALLS	25
EXTERIOR FACE OF WA	LL 40
FORMED FOUNDATION	50
NON-FORMED FOUNDAT	ION ₇₅

- 6. MINIMUM BARS SPACING :-
- CLEAR SPACING BETWEEN PARALLEL BARS SHALL NOT BE LESS THAN BAR DIAMETER OR 4/3 OF MAXIMUM AGGREGATE SIZE BUT NOT LESS THAN 25m
- CLEAR SPACING BETWEEN LAYERS OF BARS TO BE NOT LESS THAN 25mm AND THE UPPER BARS SHOULD BE OVER THE LOWER BARS
- IN COLUMNS CLEAR DISTANCE BETWEEN LONGITUDINAL BARS SHOULD BE NOT LESS THAN 1.5 BAR DIAMETER NOR LESS THAN 40mm
- 7. MINIMUM LAP LENGTH (UNLESS NOTED ON DRAWINGS) SHOULD BE AS TABLE BELWO :-

BAR DIA.(mm)	10	12	16	18	20	22	25
LAP LENGTH (mm) IN COLUMNS	400	500	600	650	700	800	900
LAP LENGTH (mm) IN ELSE WHERE	400	600	700	800	900	1000	1250

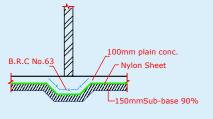
- LAP LOCATION IN SLABS AND BEAMS :-
- * AT SUPPORT FOR BOTTOM BARS.
- * AT MID SPAN FOR TOP BARS
- LAP LOCATION IN FOUNDATION :
- * AT SUPPORT FOR TOP BARS. * AT MID SPAN FOR BOTTOM BARS
- 8. VERTICAL REINFORCEMENT IN COLUMN :-
- WHERE COLUMN FACE ARE OFFSET 75mm OR MORE SPLICE OF VERTICAL BARS. TO THE OFFSET FACE SHOULD BE MADE BY SEPARATE DOWELS OVER LAP AS SPECIFIED
- WHERE A LONGITUDINAL BARS ARE OFFSET AT SPLICE THE SLOPE OF INCLINED ADJACENT PORTION SHALL NOT EXCEED 1:6 (HORIZANTAL: VERTICAL).
- CHANGING OF REINFORCEMENT BETWEEN FLOORS WHERE SUCH SITUATION OCCURS THE REINFORCEMENT OFF SHOULD BE CUT OFF AT DISTANCE 75mm BELOW FLOOR LEVEL SPACED 100mm AND PLACED BEFOR THE POINT OF BEND.
- WHERE LONGITUDINAL BARS OFFSET, PROVIDE 4TIES.
- 9. HOT & COLD WETHERING SHOULD BE ACCORDING TO ACI-305R-99
- 10. ALL REINFORCING BAR BENDS TO BE MADE COLD.
- 11. IN ONE-WAY SLAB, SHRINKAGE & TEMPERATURE REINF. STEEL EXTENDING IN THE LONG DIRECTION SHALL BE PLACED IN THE PLACE OF, AND TIED TO THE MAIN REINF. EXTENDING IN THE SHORT DIRECTION.
- 12. MIXING & PLACING CONCRETE SHOULD BE DONE ACCORDING TO ACI 318M 95 (CHAPTER 5) CONDUIT OR PIPE SIZE SHALL NOT EXCEED 30% OF SLAB THICKNESS UNLESS SPECIFICALLY DETAILED, OTHERWISE CONCENTRATIONS OF CONDUITS OR PIPES SHOULD BE AVOIDED EXCEPT WHERE DETAILED OPENINGS ARE PROVIDED, ALL SUBJECTED TO ENGINEER'S APPROVAL.



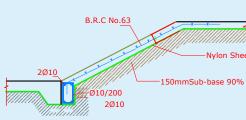
Typical Detail Of Out Door Yard



Typical Sec. For Stair On Earth

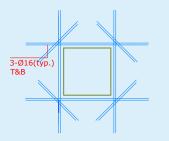


CONSTRUCTION OF PARTITION ON GROUND SLAB provid construction joint for max (5mx5m)



Typical Sec. Of Ramp

Ø10 @ 200



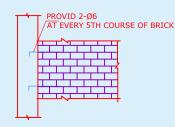
TYPICAL REINF. AROUND **OPENNINGS UP TO 600**

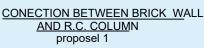


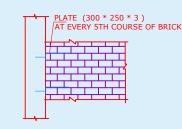
2Ø16

Ø10 @ 200

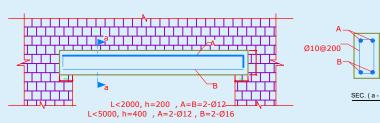
TYPICAL UP STAND DETAIL **ROOF OPENNINGS**







CONECTION BETWEEN BRICK WALL AND R.C. COLUMN proposel 2



LINTEL REINFORCEMENT

BOTT C1 CANT CANTILEVER CONSTRUCTION JOIN CL CENTRE COUL MN CONC CONCRETE DET DETAIL DIMENSION DWG DRAWING D E.A E.F DEPTH EACH EACH FACE EXPANSION JOINT ELEV ELEVATION FXP **EXPANSION** FOOTING FOOTING TYPE-1 FOUNDATION F.F.L FINISH FLOOR LEVEL GEN GENERAL GRID LINE LIVE LOAD MAXIMIN MECHANICAL MECH MIN MINIMIIM MILLIMETRES mm SEC SECTION

ABBREVIATIONS :-

BEAM

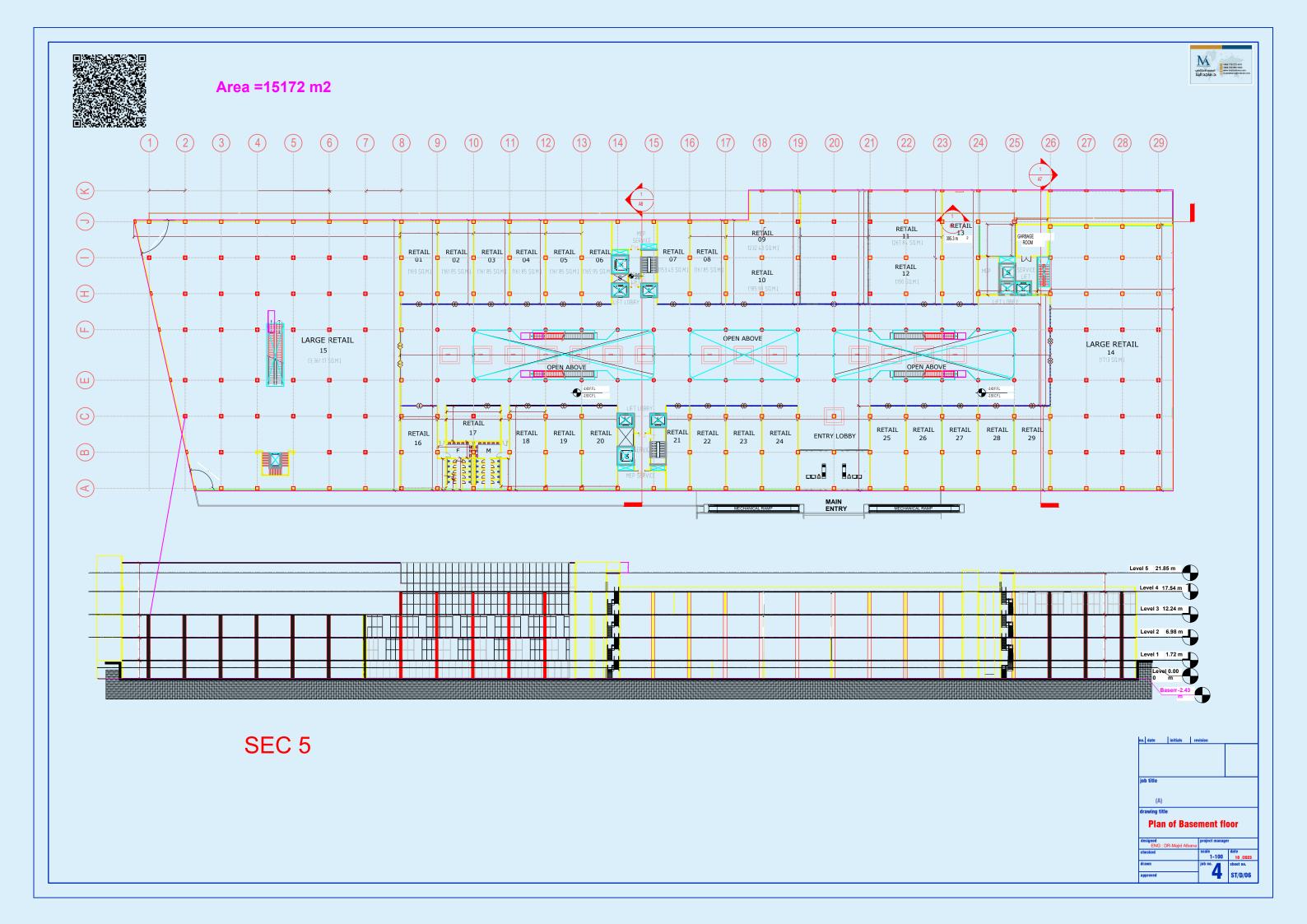
BOTTOM

ARCHITECTURAL

COLUMN TYP C1

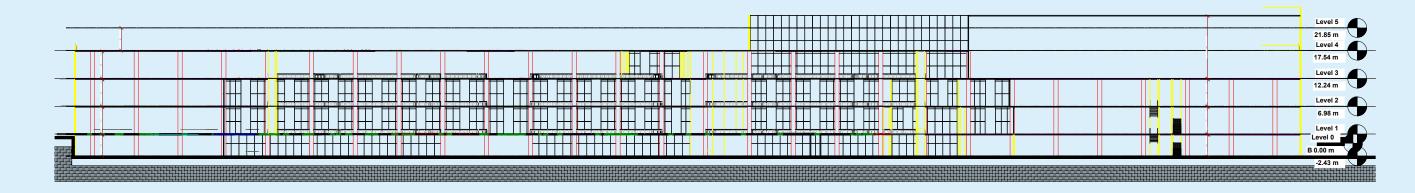
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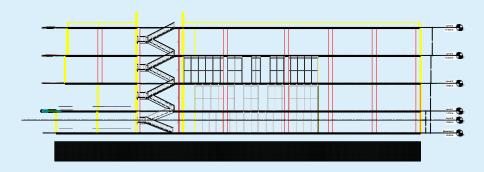


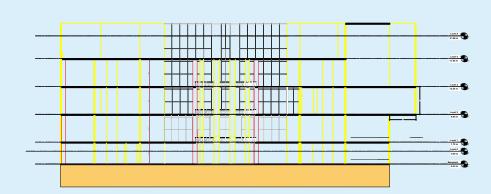






SEC6





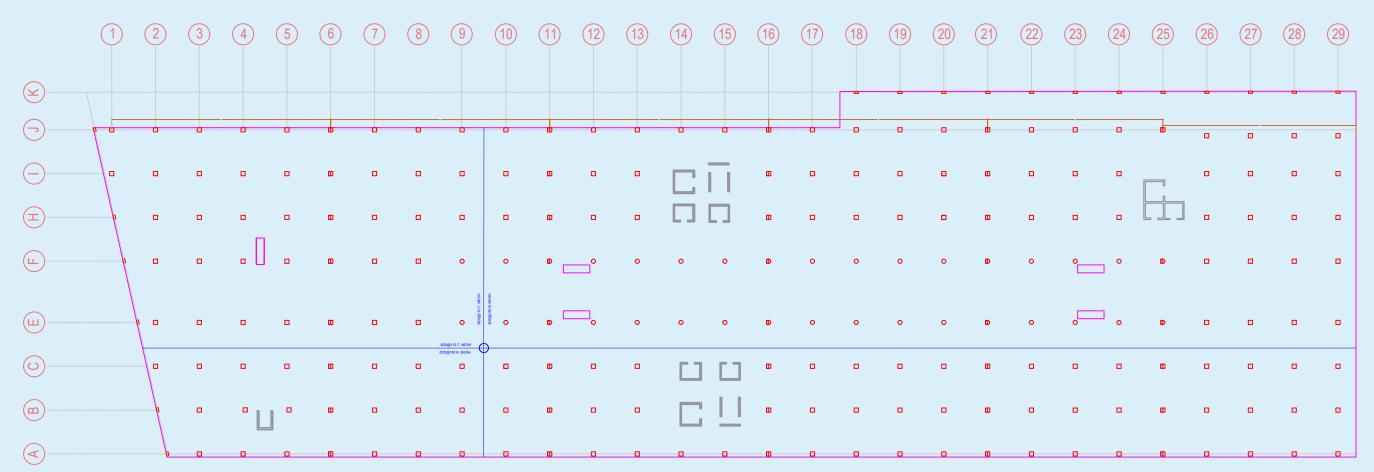
SEC7

SEC8

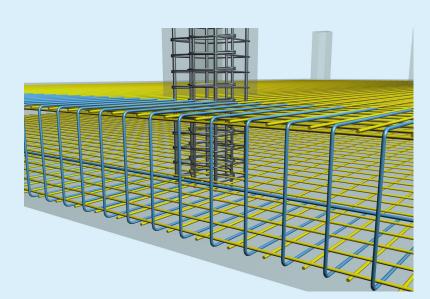












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MINIMUM LAP LENGTH (UNLESS NOTED ON DRAWINGS) SHOULD BE AS TABLE BELWO:-

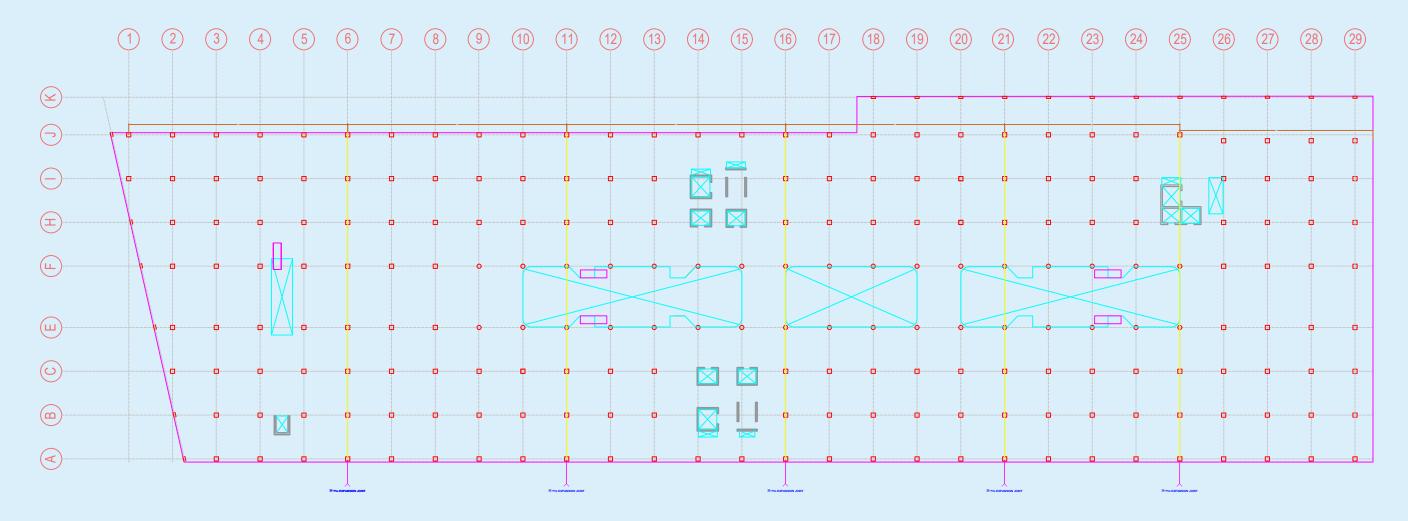
BAR DIA.(mm)	10	12	16	18	20	22	25
LAP LENGTH (mm) IN COLUMNS	400	500	600	650	700	800	900
LAP LENGTH (mm) IN ELSE WHERE	400	600	700	800	900	1000	1250

THICK. = 1000 mm



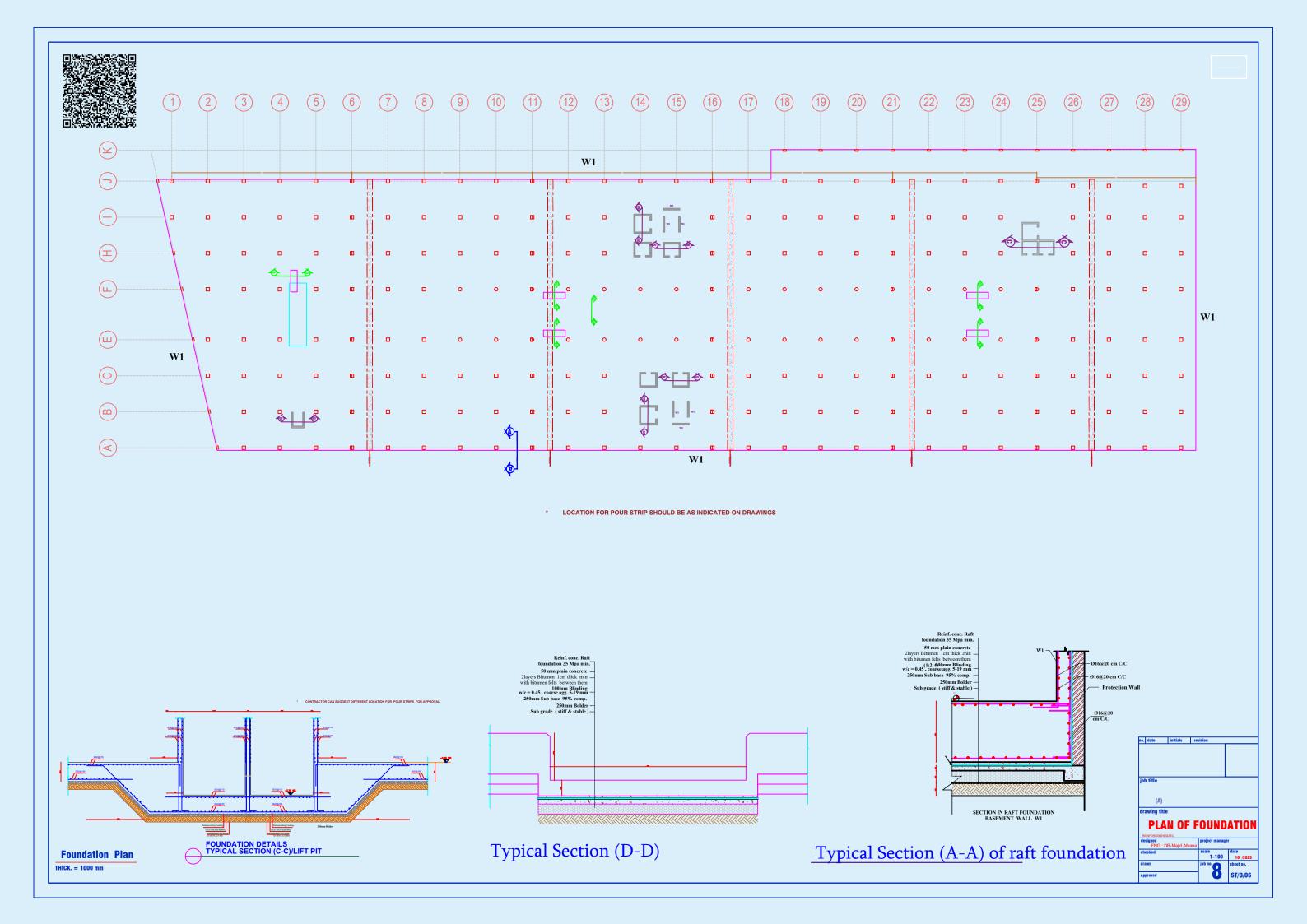


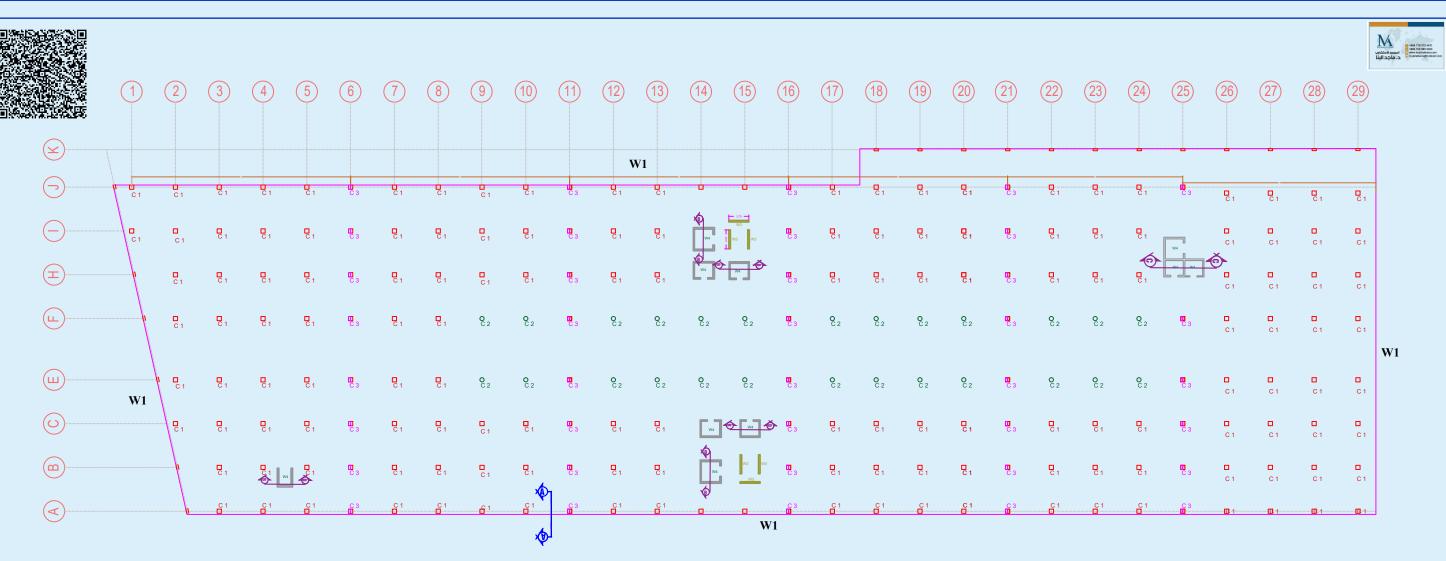




Foundation Plan / EXPANSION JOINT

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Notes

-Feu = 45 N/mm2 -Fy = 420 N/mm2.

CONCRETE COVERS

-SLABS = 25 mm
-BEAMS = 40 mm
-COLUMNS = 40 mm
-WALLS = 25 mm
-SLAB ON GRADE = 50 mm
-RAFT FOUNDATION = 75 mm

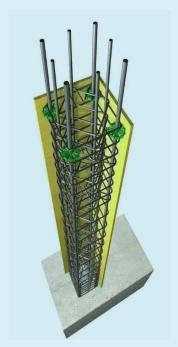
-THE GEOTECHNICAL THE BEARING

CAPACITY OF THE SOIL = 100 kN/m2

-THE BUILDING IS DESIGNED FOR

BASEMENT + GROUND FLOOR +3 FLOORS

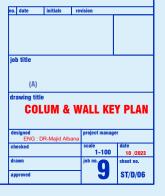
+ PENT-HOUSE

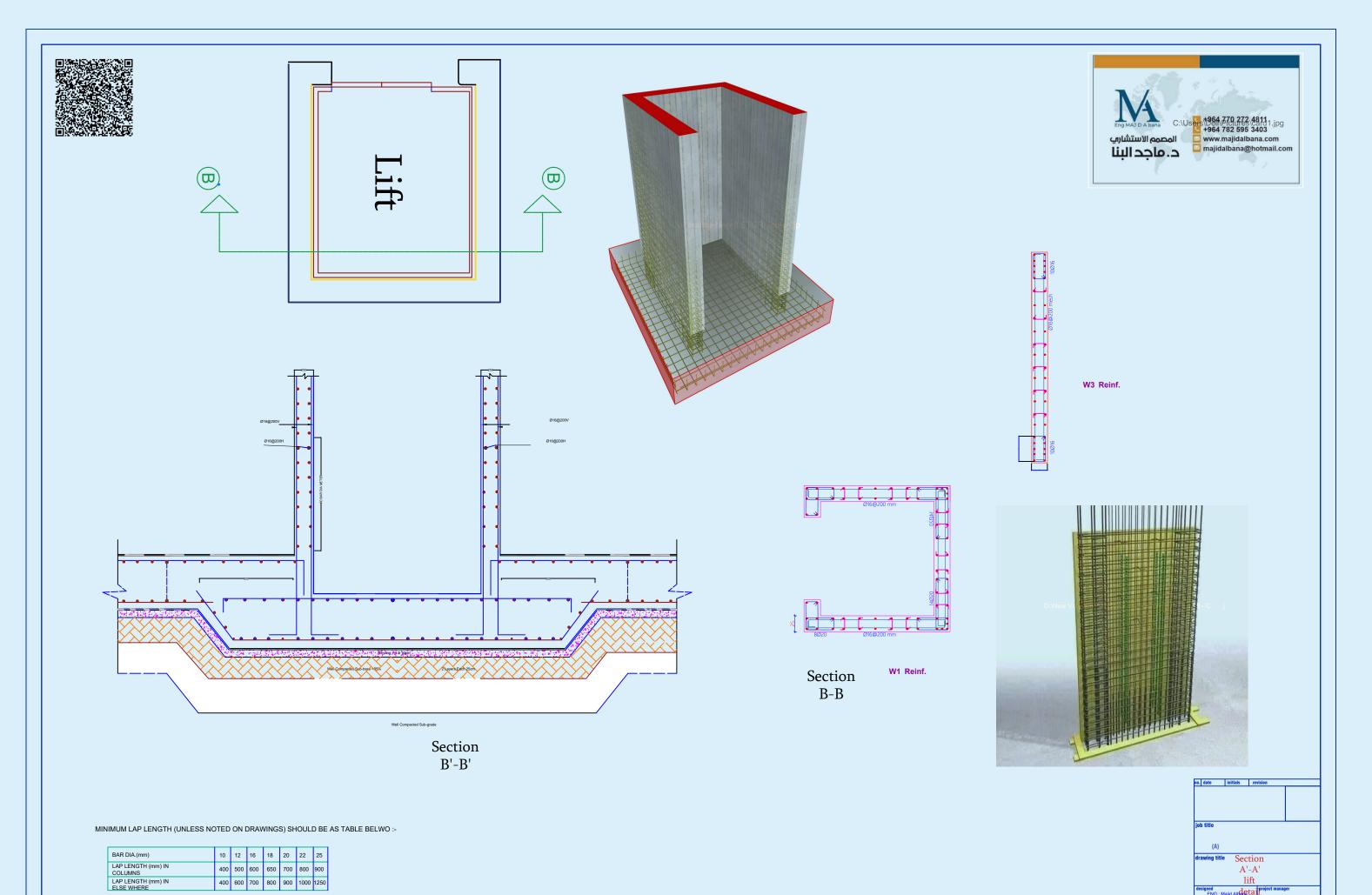


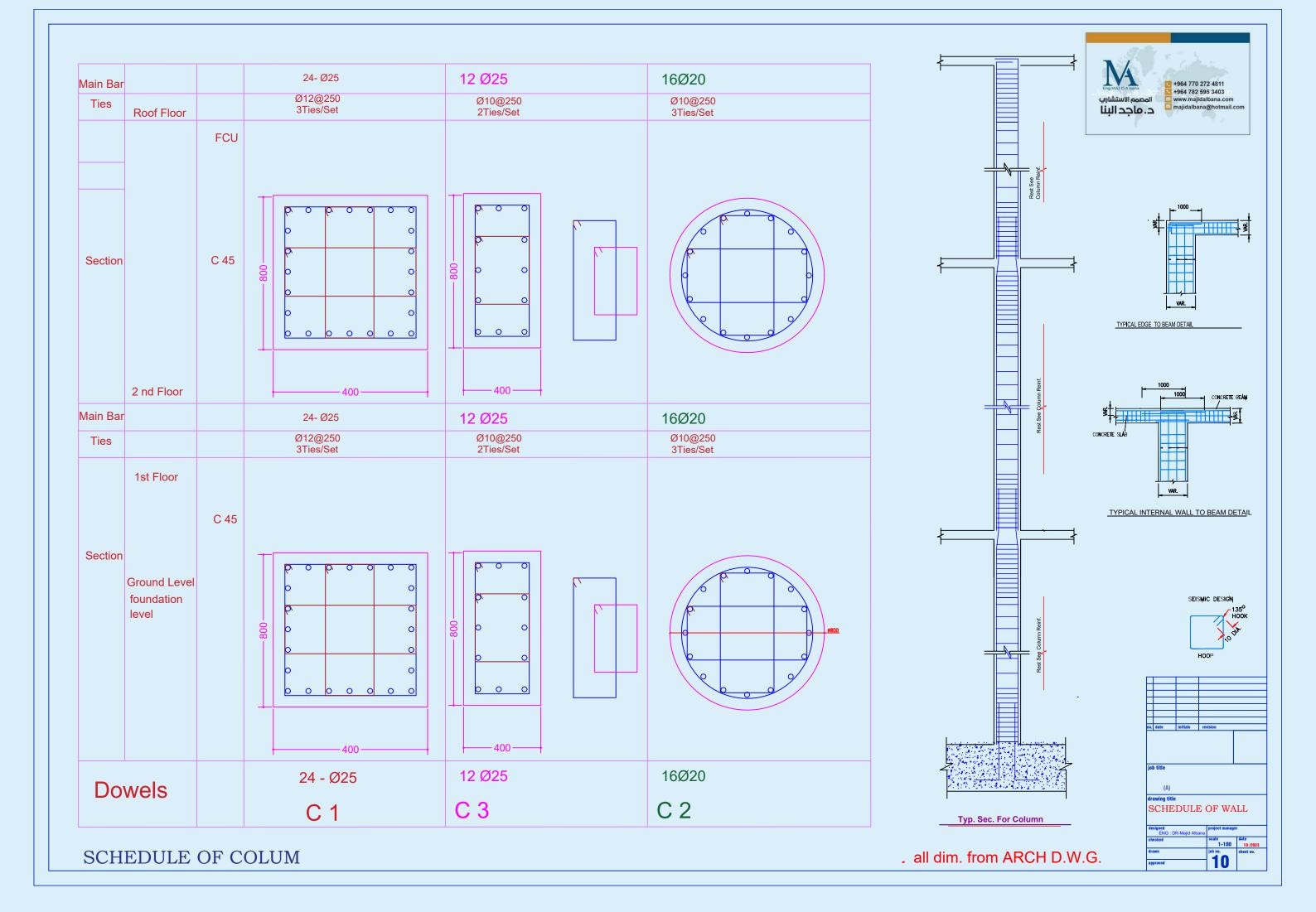
SCHEDULE OF COLUMNS AND WALLS

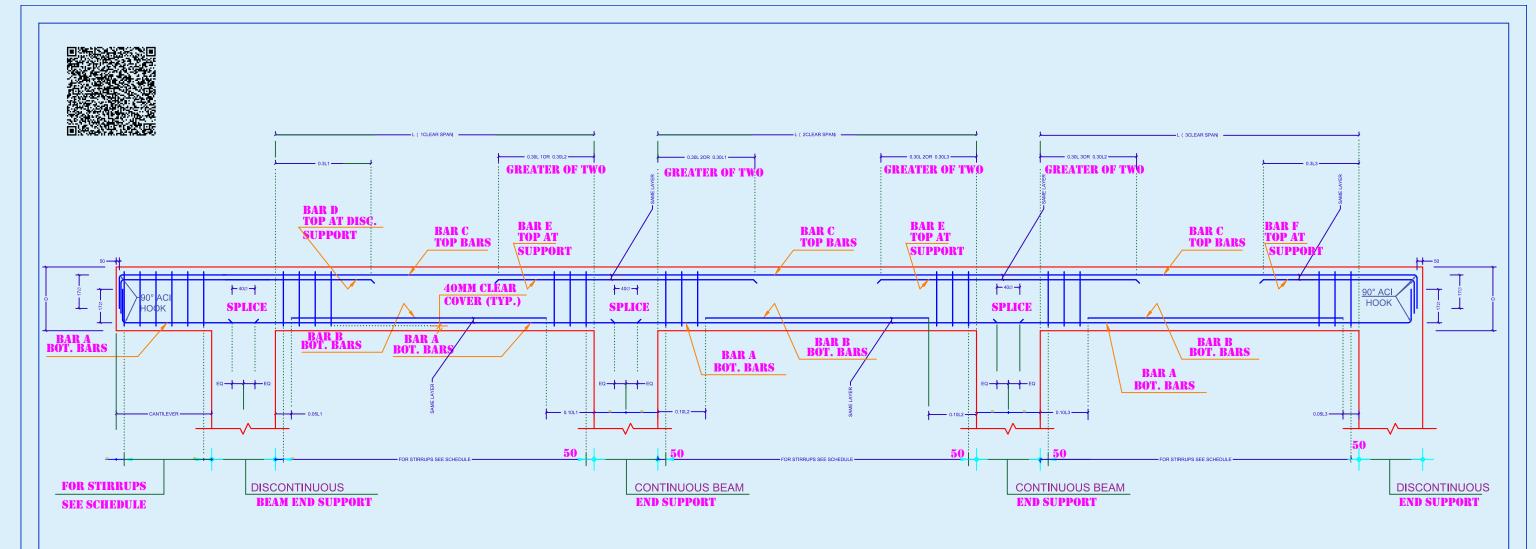
COLUMNS	S	SIZE	
OR WALLS ID	(mm)	WIDTH (mm)	REMARK
C1	800	800	
C2	800		
C3	800	400	
W3	dwg.	300	
W4	dwg.	dwg.	lift

COLUM & WALL KEY PLAN







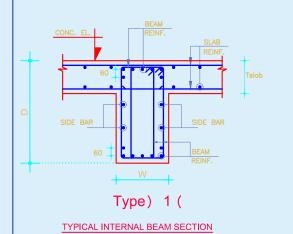


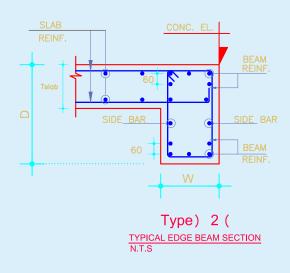
NOTES:

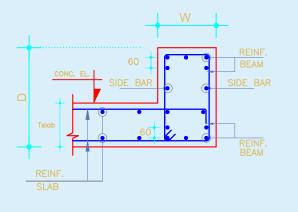
- 1. REFER TO BEAM SCHEDULE FOR NO. AND SIZE OF TOP AND BOTTOM BARS REQUIRED PER BEAM.
- PROVIDE 60MM CENTRE TO CENTRE WHEN SCHEDULES CALL FOR 2 LAYERS OF REBARS.
 FOR CANTILEVER BEAMS OR RIBS, BARS SHOULD BE EXTENDED UP TO ONE HALF THE CANTILEVER SPAN.

TYPICAL BEAM LONGITUDINAL SECTION

NOT TO SCALE

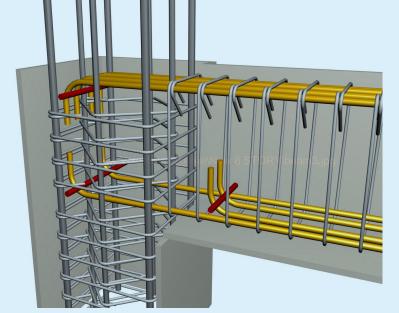


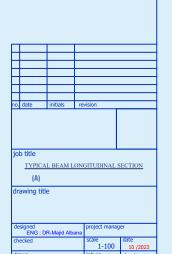


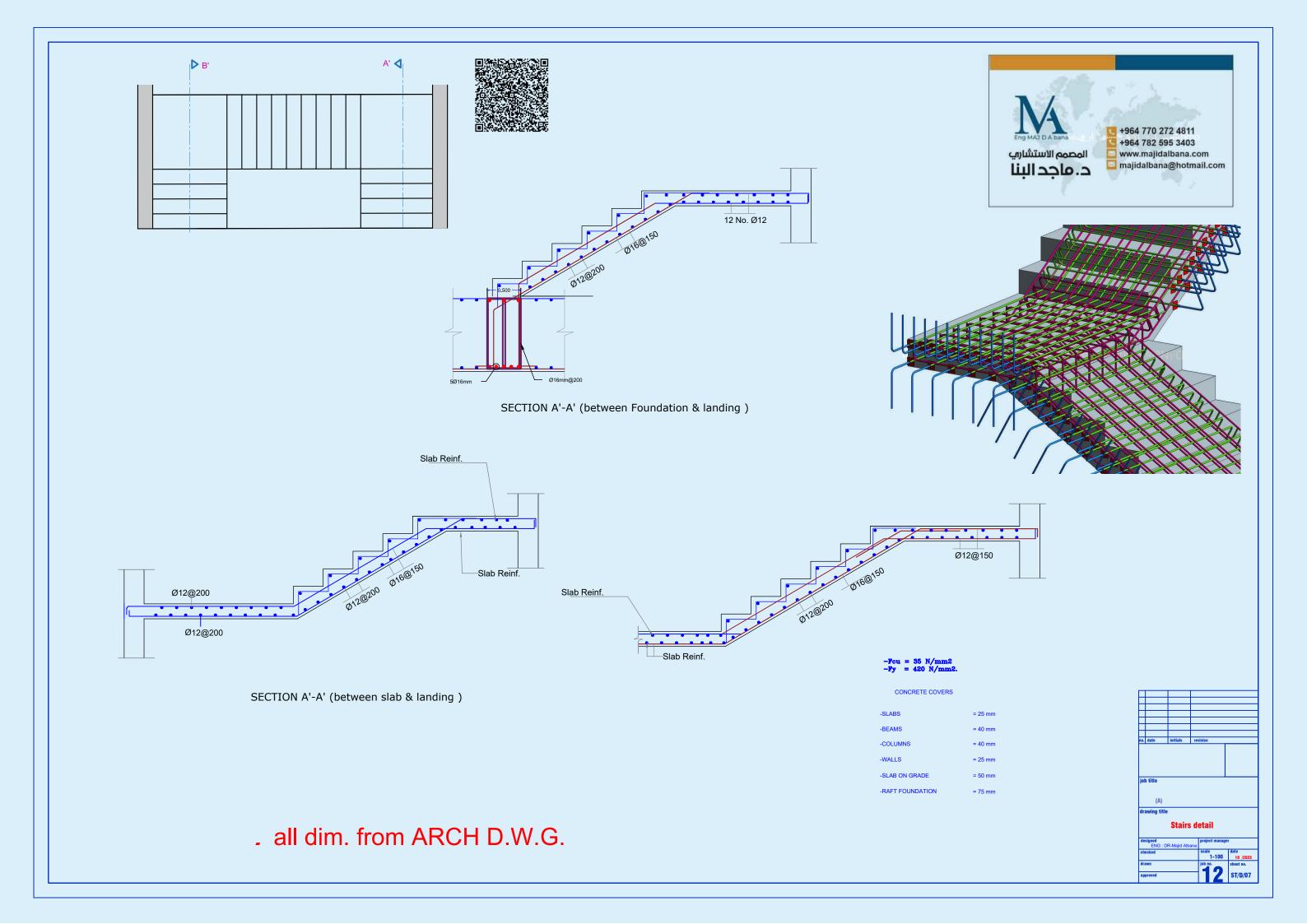


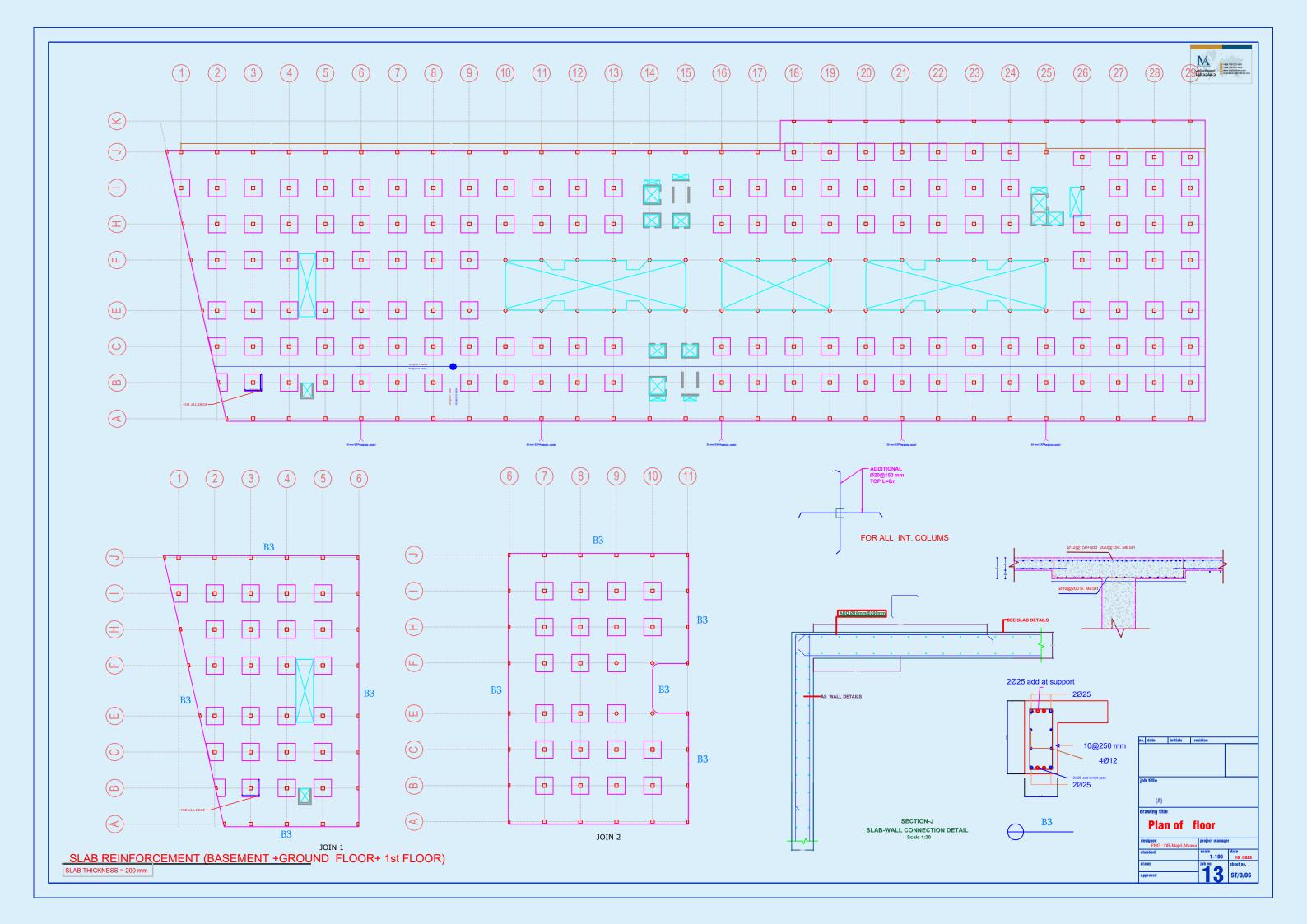
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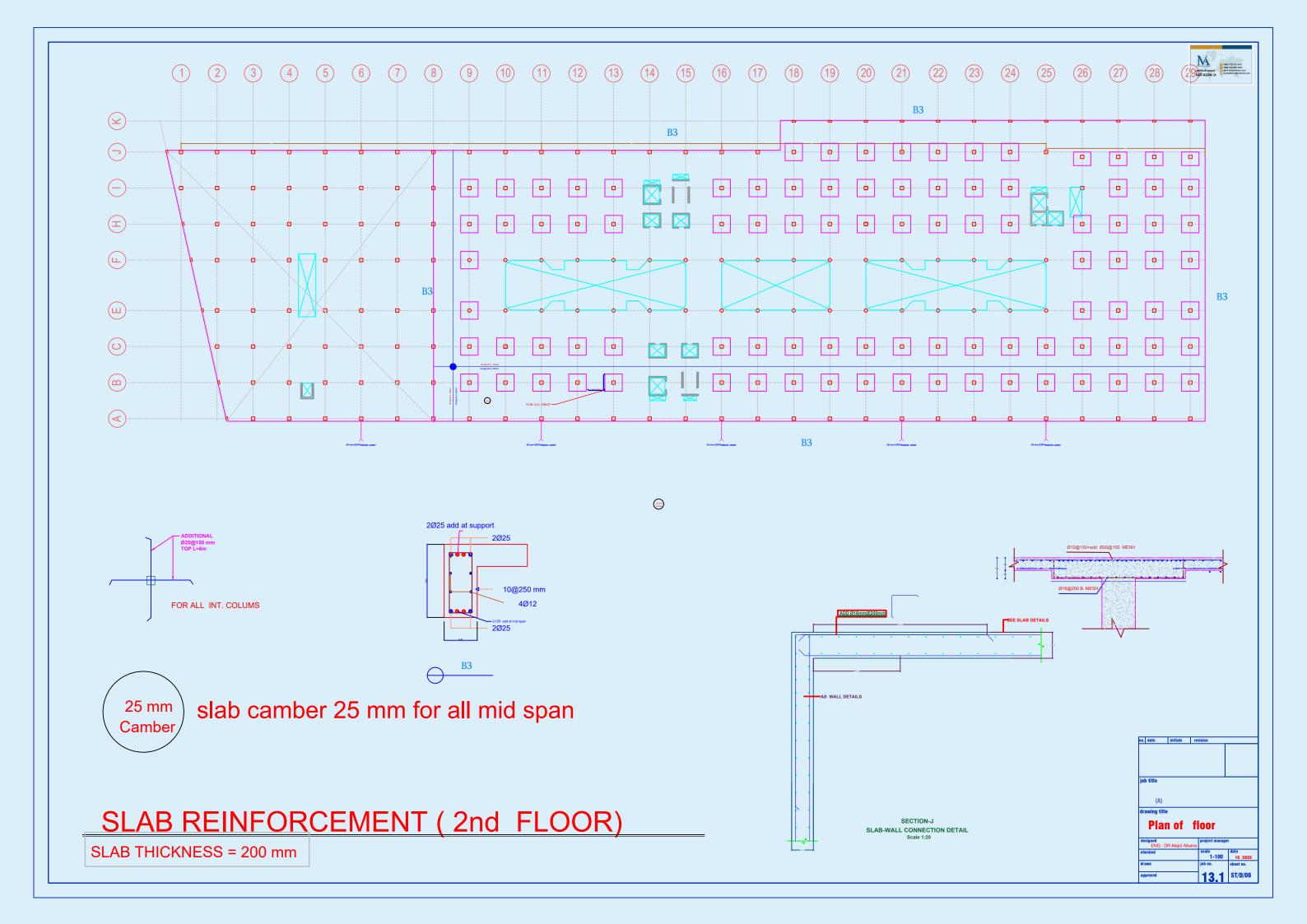
TYPICAL EDGE INV. BEAM SECTION N.T.S

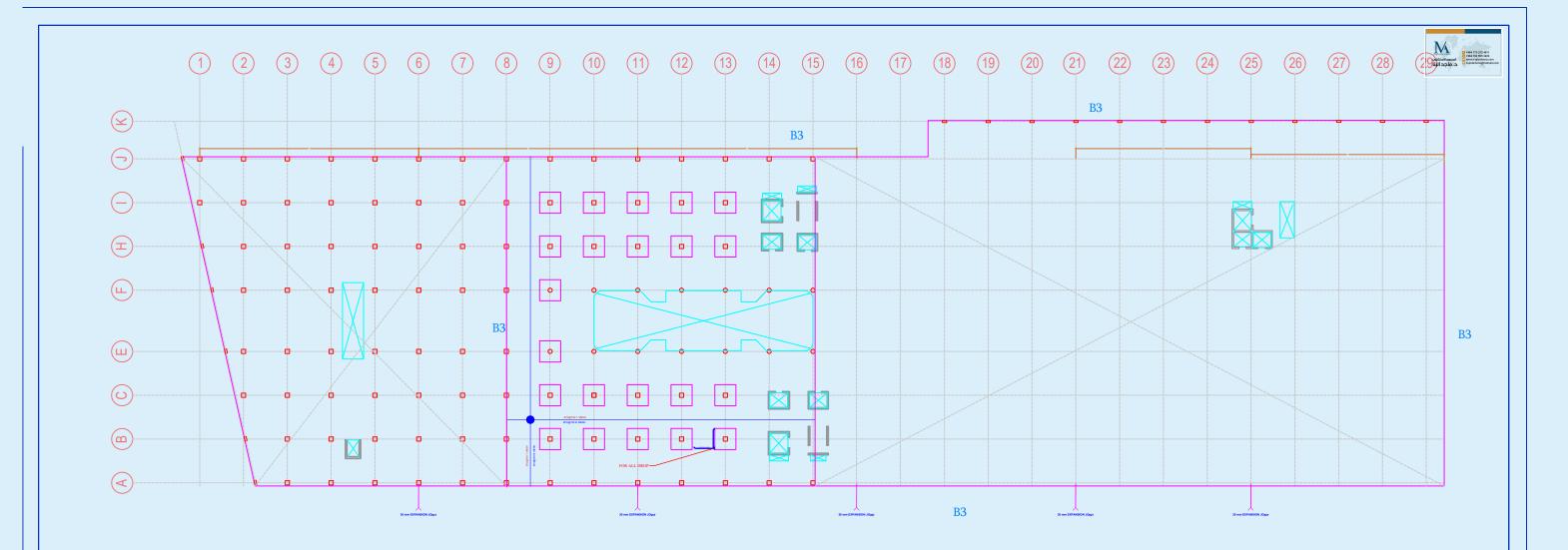




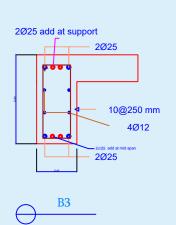






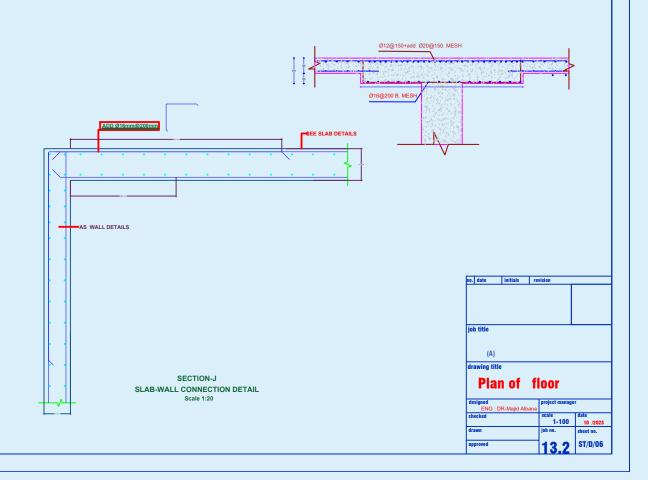






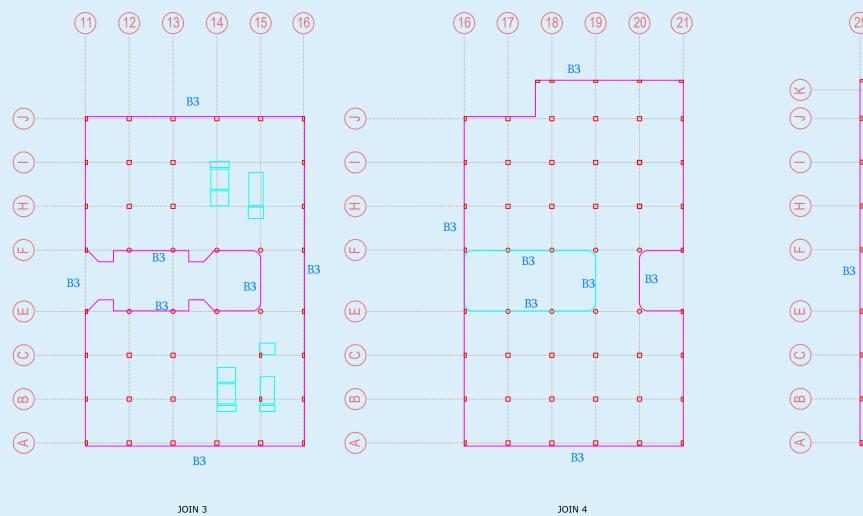
SLAB REINFORCEMENT (2nd FLOOR)

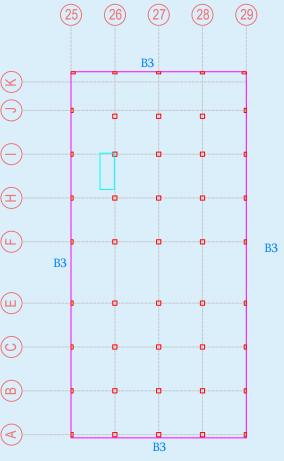
SLAB THICKNESS = 200 mm







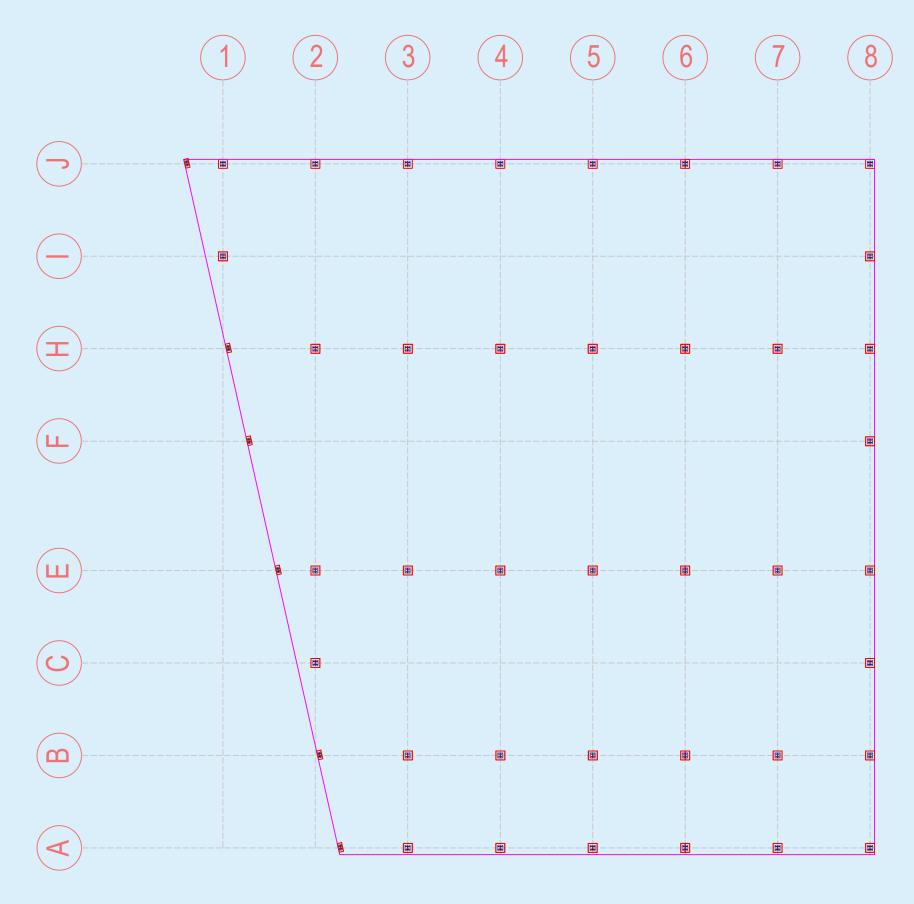




JOIN 4 JOIN5









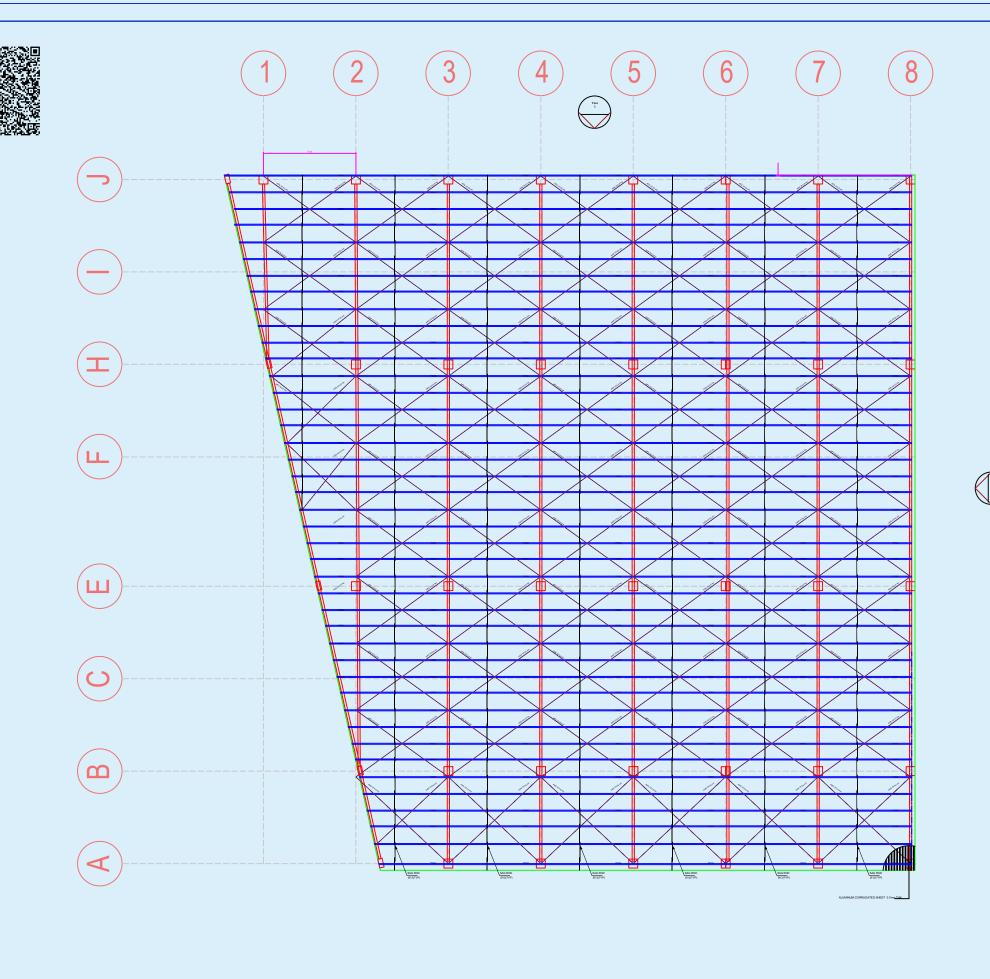
STEEL MATERIALS

- Material Steel S235Minimum yield stress Fy = 235 MPaMinimum Tensile Strength Fu = 360 MPa
- 2 All Steel Members shall Be Hot-Dipped
 Galvanized Including Plates , Bolts <nuts
- 2 All Welding, Unless Otherwise Specified, Shall Be Fillet or Full Welded All Round With Weld Size Equal to The Thickness of Respective Welded Member
- 3 All Welds Are 8mm E7018

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COLUMN AND BASE PLATE LAYOUT

TOP FOUNDATION LEVEL +12.24





STEEL MATERIALS

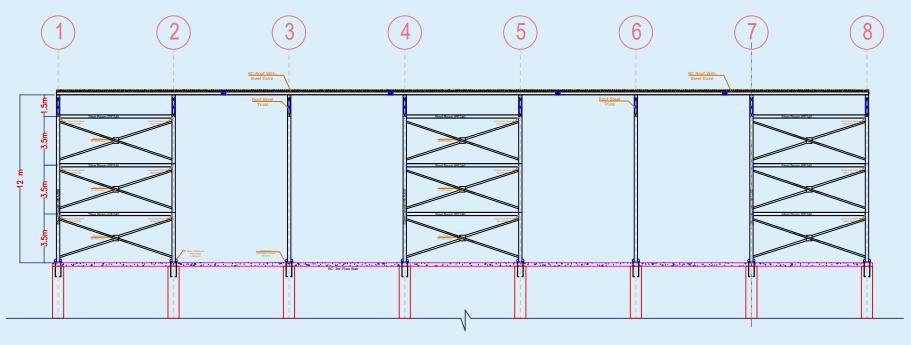
- Material Steel S235Minimum yield stress Fy = 235 MPaMinimum Tensile Strength Fu = 360 MPa
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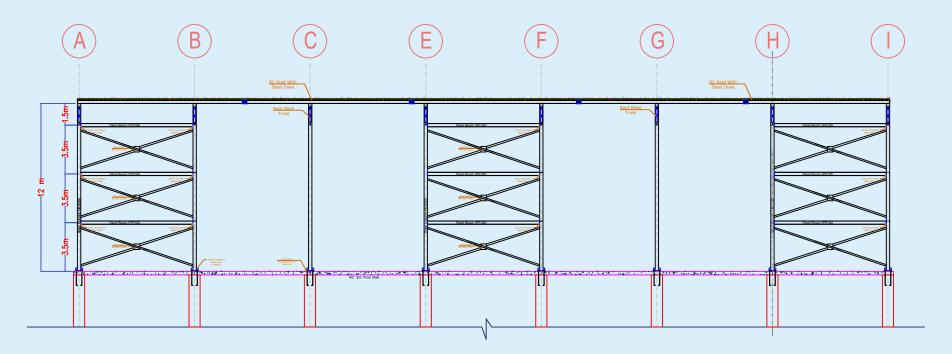
SLAB LAYOUT

TOP SLAB LEVEL +24.24





Steel Structure Building - View 1



Steel Structure Building - View 2

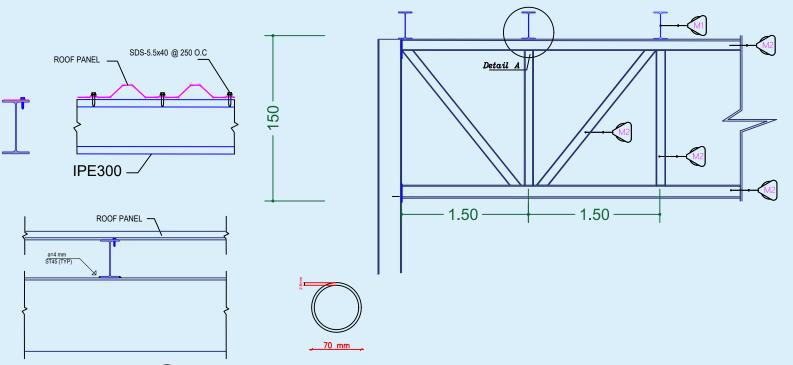


STEEL MATERIALS

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- 2 All Steel Members shall Be Hot-Dipped
 Galvanized Including Plates , Bolts <nuts
- 2 All Welding, Unless Otherwise Specified, Shall Be Fillet or Full Welded All Round With Weld Size Equal to The Thickness of Respective Welded Member
- 3 All Welds Are 8mm E7018

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PIPE 70*2.6 mm

BASE PLATE



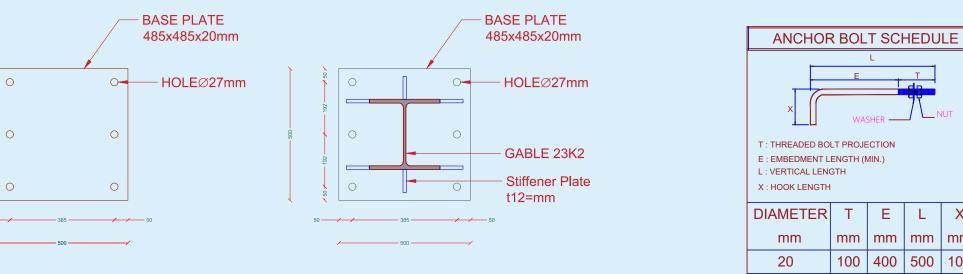
TRUSS MEMBER DETAILS

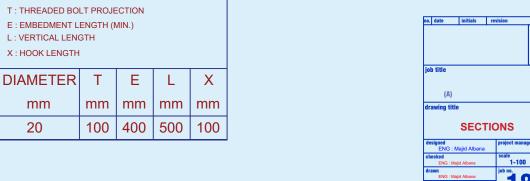
STEEL GRADE

ST37 (TYP)

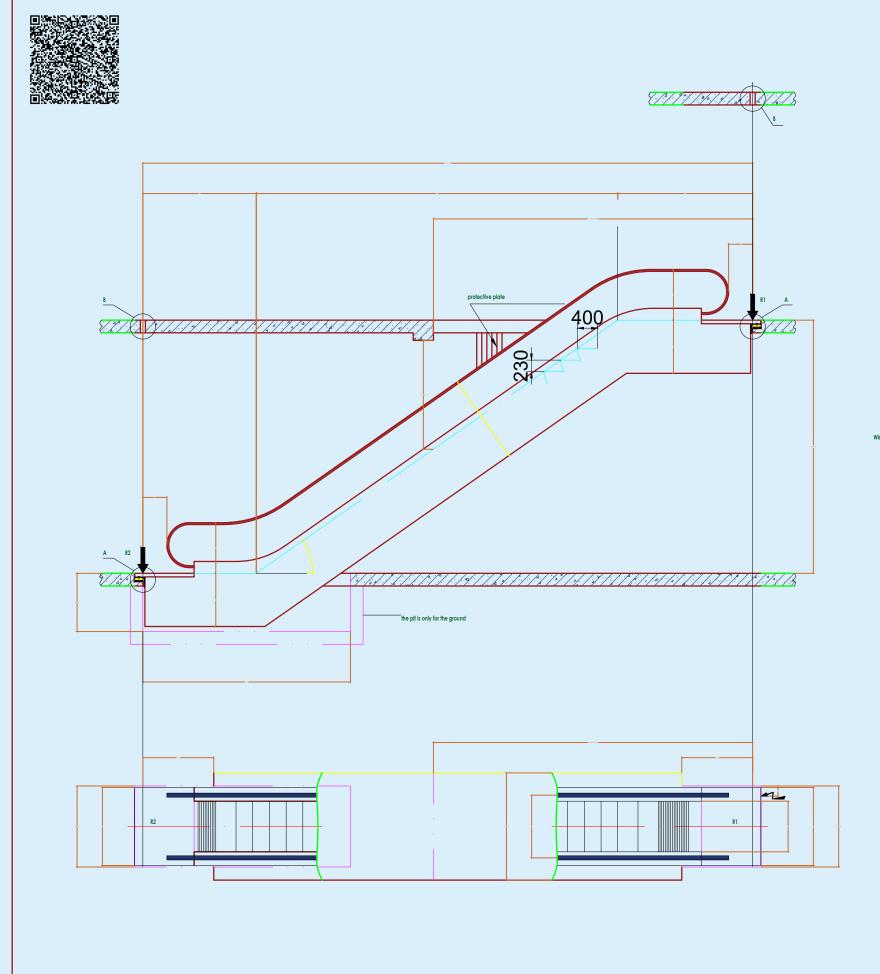
STEEL MATERIALS

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- 2 All Steel Members shall Be Hot-Dipped
 Galvanized Including Plates , Bolts <nuts
- 2 All Welding, Unless Otherwise Specified, Shall Be Fillet or Full Welded All Round With Weld Size Equal to The Thickness of Respective Welded Member
- 3 All Welds Are 8mm E7018

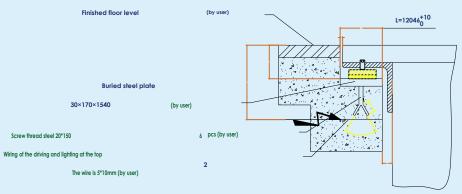








Detail A

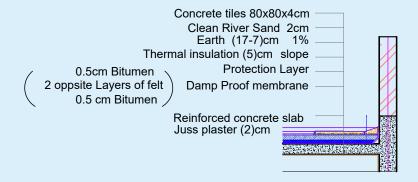


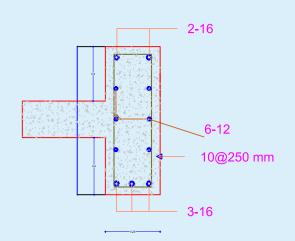
Detail B

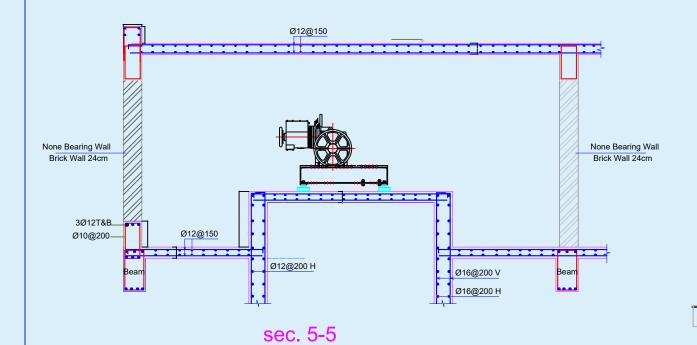


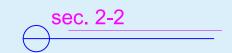








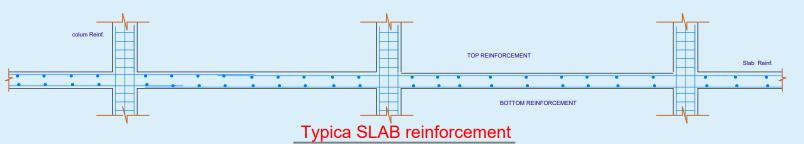






SLAB REINFORCEMENT (PEN HOUSE FLOOR)

SLAB THICKNESS = 250 mm



MINIMUM LAP LENGTH (UNLESS NOTED ON DRAWINGS) SHOULD BE AS TABLE BELWO :-

BAR DIA.(mm)	10	12	16	18	20	22	25
LAP LENGTH (mm)CMSLUMNS						800	
LAP LENGTH (mm\$LMB & BEAN	13400	600	700	800	900	1000	1250

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